

A High-Accuracy Empirical Formula for the Strain Concentration Factor in Countersunk Holes

Mohammad A. Gharaibeh

Department of Mechanical Engineering, Faculty of Engineering, Hashemite University, Zarqa 13133, Jordan

Corresponding Author Email: mohammada_fa@hu.edu.jo



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ABSTRACT

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This paper presents a modified high-accuracy empirical formula for the strain concentration factor in a centrally-placed countersunk holes in isotropic plate under uniaxial tension. Finite Element Method (FEM) was used to investigate the effect of the problem geometric parameters including, plate width and thickness, as well as the hole radius, countersinking depth and angle on the strain concentration factor. The important influence of Poisson's ratio was also thoroughly discussed. Based on the FEM-generated data and nonlinear regression, a general and high-precision equation for the strain concentration factor was developed. The formulation process was based on producing a general formula for computing the strain concentration factor with unknown coefficients. Such coefficients are determined by minimizing the relative error between the fitted equation and the FE data using nonlinear least squares method. The results of this newly-developed equation were validated with FEA. The comparison showed high accuracy of the present equation in evaluating strain concentration factor in countersunk holes with a relative approximate error of less than 7%. Besides, this equation was efficiently employed to test the various geometric and material parameters on the strain concentration value of countersunk holes. The results of the present equation were compared to the results of older equation available in literature. The comparison proved much higher accuracy of the present equation in evaluating strain concentration factor especially for deeper and larger countersunk holes than the previously published formula.

1. INTRODUCTION

In structural engineering, riveting is a very popular joining method of mechanical and structural components such as elastic beams and elastic plates. Countersunk holes are common footprint of rivet which forms geometric discontinuities throughout the thickness of the structure, i.e. plate. This kind of holes generates complicated high stress and strain distributions and produces regions of localized high stress and strain which are commonly referred to as stress and strain concentration regions. An accurate analysis of such high local stresses and strains is very crucial in predicting joint strength and hence fatigue life of the structural member. Stress and strain concentrations are typically measured by the means of stress concentration factor (SCF) and strain concentration factor ($S\epsilon CF$). Such factors are generally defined as the ratio between the maximum stress/strain and the average (nominal) stress/strain values. In fact, several machine design books [1] have insisted on the essential need of reliable strain concentration factor computations, as there are failure theories, such as the maximum strain theory, are mainly focused on the maximum principal strain values in the loaded member. Interestingly, stress concentration factor (K_t) and strain concentration factor ($K_{t,\epsilon}$) are mathematically related. Specifically, $K_{t,\epsilon} = K_t$ for plane stress and $K_{t,\epsilon} = (1-\nu^2)K_t$ for plane strain problems, where ν is the Poisson's ratio of the material of the structure with the stress/strain riser, as proved by Dowling [2].

Stress concentration factors has been widely studied. Pilkey et al. [3], and Savin [4] reported numerous data on the SCF in different stress risers under various loading conditions. Shivakumar and Newman Jr. [5, 6] studied SCF in circular holes drilled in thin and thick elastic plate under uniaxial tension using finite element analysis (FEA). Their studies showed that the stress concentration occurs near the plate edge, in both thin and thick plate systems, perpendicular to the loading direction. However, for thick plates, the high stress localization was found to be at the mid-thickness of the plate and it decays at the top and bottom free surfaces. Another SCF study for plates with central circular drill holes and uniaxial and biaxial tensile loadings was conducted by Mu and Wu [7]. Several research studies in literature for the investigation of SCF in centrally-placed circular holes are available [8-12]. More data on the calculation of SCF in elliptical holes and other types of cutouts is also available [13-16].

For the investigation of SCF and in countersunk holes, only a few studies and papers are available. The first experimental study was performed by Whaley [17]. In which he studies stress distributions at the top and bottom surfaces of the hole instead of its interior. Cheng [18] used stress freezing method to experimentally obtain stresses in countersunk holes through the thickness of the plate. In his study, he considered several countersink depths and angles and two loading conditions: tensile and bending. His stress results showed that the high stress localization occurs at the edge of the countersink. Shivakumar et al. [19, 20] and Bhargava et al. [21] conducted

a thorough FEA investigation for the SCF in countersunk holes under remote tension. They also provided empirical formulations to calculate SCF in countersunk holes with different geometric properties. Later, Darwish et al. [22, 23] and Gharaibeh et al. [24] proved that the error in the equations provided by Bhargava et al. [24] is considerably high for thick countersunk hole with large radii and countersink depths. As a result, they provided modified and more accurate equations to compute SCF in countersunk holes subjected to uniaxial tension.

For the S_εCF, Chaudhun et al. [25] investigated a long-range elastic strains around circular holes and other discontinuities using X-ray topography methods. Pandita et al. [26] investigated the strain field in orthotropic plates with centrally circular and elliptical holes due to tension using digital photogrammetry. Their results showed that strain concentration values are affected by the loading direction and the holes geometry and dimensions.

Yang et al. [27] used finite element method (FEM) to study stress and strain fields around circular holes in finite thickness large plates. The numerically-simulated results revealed that the stress and strain concentrations occur at the mid-plate of the thickness in thin plates and their location vary in thick plates. Besides, they claimed that the S_εCF is often related to Poisson's ratio; especially for thick plate systems. Ray-Chaudhuri and Chawla [28] investigated the strain concentration in various hole shapes in orthotropic composite plates under uniaxial tension using FEA. The simulation results proved that strain concentrations are highly affected by the shape, size and eccentricity of the hole in addition to the number of plies in the composite plate, fiber orientation and the curvature of the plate. Recently, Ball et al. [29] studied both elastic and plastic stress and strain responses of circular and V-shaped notches in flat plates using FEA and experiments of surface differential displacement mapping. Their results showed that larger and deeper notches end up in higher stress and strain values around the discontinuity. Besides, they investigated the accuracy of their FEA models compared to experimental findings. Zhu et al. [30] proposed a correction factor that includes the effect of Poisson's ratio effect to quantify the S_εCF induced in notched plates using nonlinear finite element computations. Then, they used the results of the new method to fit a high accuracy fatigue life prediction models of TC4 and GH4169 alloyed plates. Guo, W. and Guo, W.L. [31] obtained a complete set of empirical formulas to compute 3D SCF and S_εCF in plates with finite thickness having central circular and elliptical holes under uniaxial tension. Additionally, the computations of the empirical formulas were validated with FEA data and other available theoretical solutions in literature. Tlilan et al. [32] introduced a study on the S_εCF of thick-walled cylinders with internal pressure. In their study, triaxial as well as biaxial stress states were considered in both open and closed end vessels. The FEA results revealed that the maximum S_εCF is located at the inner surface of the pressurized cylinder. Also, the S_εCF decreases throughout the thickness toward the outer surface of the pressurized vessel.

For S_εCF in countersunk holes, Hayajneh et al. [33] presented strain concentration FEA study in countersunk holes in orthotropic plates under tension. In their FEA models, they adopted two modeling strategies, homogenous modeling and ply-by-ply modeling methods. The results showed that S_εCF is function of the hole geometry as well as plate material properties, i.e. Poisson's ratio. Bhargava and Shivakumar [34]

formulated an empirical equation for the S_εCF in countersunk holes in isotropic plate subjected to tensile loading based on finite element analysis data. However, it was found that for holes with larger radius and countersink depths in thicker plates, the equations in Ref. [34] are inaccurate and they could lead to erroneous S_εCF results.

The objective of this paper is to formulate an accurate equation to compute the strain concentration factor ($K_{t,\epsilon}$) developed in countersunk holes subjected to uniaxial tension. This proposed equation is more accurate than the equation presented by Bhargava and Shivakumar [34] especially for larger and deeper countersunk holes as the presently proposed equation includes the interaction effect between the hole radius to plate width ratio and the countersink depth which was previously neglected in Bhargava's work. Specifically, the maximum computed error with respect to FEA data was reduced from 17% to 5%. Therefore, the ignorance of such important effects of the larger and deeper holes could lead to erroneous ($K_{t,\epsilon}$) values and hence false and extremely dangerous engineering designs.

2. CONFIGURATION AND MATERIAL

The geometric configuration of a plate with a central countersunk hole is presented in Figure 1. This figure defines the following geometric parameters: The plate length ($2h$), width ($2w$) and thickness (t). The plate thickness consists of the straight shank thickness (b) and the sinking depth (C_s), therefore ($t=b+C_s$). Also, the straight shank radius is (r) and the countersink angle is (θ_c). In practice, the countersink angle is commonly available in the range of 80°-120°.

In the present analysis, a homogenous, isotropic and linear elastic properties of the Aluminum were used for the plate system with a modulus of elasticity of ($E=70$ GPa) and Poisson's ratio of ($\nu=0.3$). For such material configuration, the stress and strain concentration values are independent of E . However, Poisson's ratio value effects the strain concentration and this effect ranges from 1 for plane stress state to $1-\nu^2$ for plane strain condition.

For a loaded structure, stress and strain concentrations are often defined as the localization of high stress or strain that is results from geometric discontinuities like holes and notches. An accurate analysis of such high local stresses and strains is very crucial in predicting the strength and hence fatigue life of a structural member. Stress and strain concentrations are typically represented by the means of stress concentration factor (SCF) and strain concentration factor (S_εCF). Such factors are generally defined as the ratio between the maximum stress/strain and the average (nominal) stress/strain values. Interestingly, stress concentration factor (K_t) and strain concentration factor ($K_{t,\epsilon}$) are mathematically related. Specifically, $K_{t,\epsilon}=K_t$ for plane stress and $K_{t,\epsilon}=(1-\nu^2)K_t$ for plane strain problems, where ν is the Poisson's ratio of the material of the structure with the stress or strain riser.

In general, the main focus of this paper is the strain concentration factor ($K_{t,\epsilon}$) which is mathematically defined as:

$$K_{t,\epsilon} = \frac{\epsilon_{max}}{\epsilon_{nom}} \quad (1)$$

where, ϵ_{nom} is the nominal strain and ϵ_{max} is the maximum strain. In the present configuration, the maximum strain is located at the countersink edge and the nominal strain is equal

to the applied strain (ϵ_o). It is important to mention that both ϵ_{nom} and ϵ_{max} in the y -direction, as shown in Figure 1.

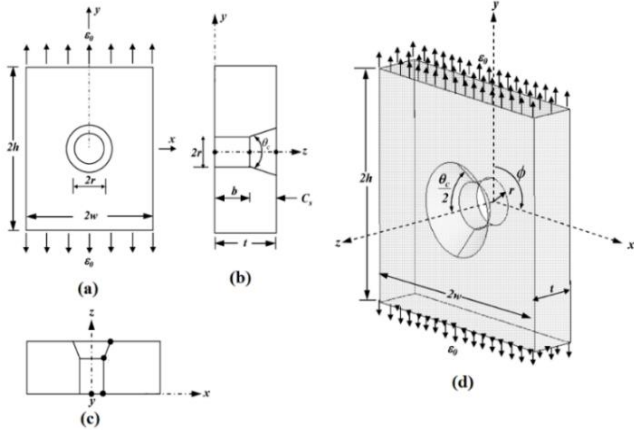


Figure 1. Configuration of the countersunk hole: (a) x - y plane (b) y - z plane (c) x - z plane and (d) 3D configuration

In this paper, a comprehensive finite element analysis is performed to investigate the SeCF in countersunk holes at different countersink angles (θ_c), thickness to radius ratio (t/r), countersink depth to plate thickness ratio (C_s/t) and radius to width ratio (r/w). To eliminate the influence of the plate length on $K_{t,\epsilon}$, the plate length to radius ratio is kept constant at ($h/r=15$) throughout the analysis. Using the FEA data and using nonlinear regression, an empirical formula for $K_{t,\epsilon}$ is developed and hence validated. The formulation process was based on producing a general formula for computing the strain concentration factor with unknown coefficients. Such coefficients are determined by minimizing the relative error between the fitted equation and the FE data using nonlinear least squares method.

3. FINITE ELEMENT MODELING

The finite element model of the isotropic plate with a central countersunk hole used in this work was built using ANSYS R17.1. Per the symmetry of the present problem, only one symmetric quarter model was considered. Symmetric boundary conditions were imposed by setting the displacement in x and y directions to zero on $x=0$ and $y=0$ planes, respectively. Additionally, the out-of-plane displacement (u_z) was constrained at a single node located at $x=h$, $y=w$ and $z=0$. A remote strain ($\epsilon_o=1$) was applied on the $x = h$ plane using INISTATE command available in ANSYS. This quarter symmetric model is shown in Figure 2.

For the FEA mesh design, only three-dimensional hexahedron elements, defined as SOLID185 in ANSYS, was adopted to generate the mapped FEA mesh. Additionally, care was taken to have finer mesh near the hole and relatively coarser mesh elsewhere. This was done ensure best strain solution accuracy at the area of interest with minimum FE model solution time. Furthermore, to optimize model solution time and strain results accuracy, mesh sensitivity study was performed. In this sensitivity study, five FE models with different mesh characteristics were tested, as listed in Table 1. For each mesh model, the $K_{t,\epsilon}$ value was recorded and compared to the value of the subsequent mesh model. This study was conducted for $t/r=2$, $C_s/t=0.4$ and $w/r=l/r=15$ at $\theta_c=100^\circ$ geometric configuration.

Table 1. Mesh density study FE mesh details

Model #	Number of elements	Number of nodes	Radius-to-element size ratio at the hole (r/e)	$K_{t,\epsilon}$
1	1380	1848	12.7	3.8231
2	5100	6160	19.0	3.8842
3	25200	28158	25.3	3.8938
4	48960	53475	38.0	3.8941
5	144000	152971	44.3	3.8943

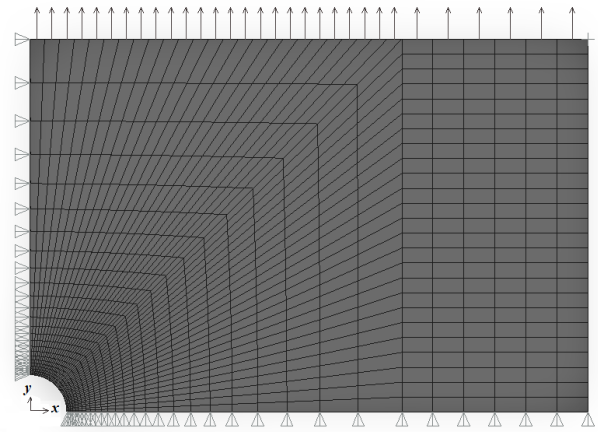


Figure 2. Loading and boundary conditions applied on the symmetric quarter model

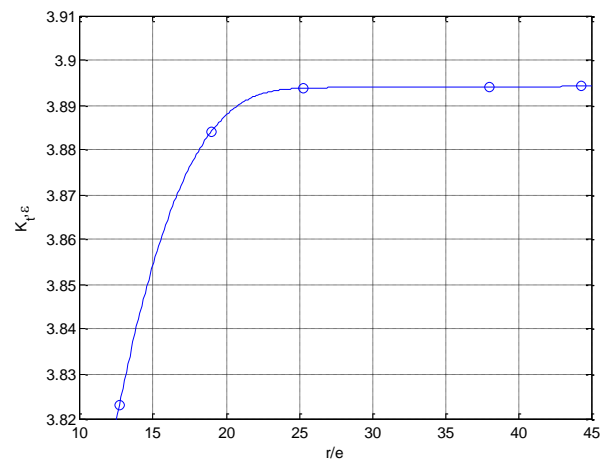


Figure 3. Mesh sensitivity study results

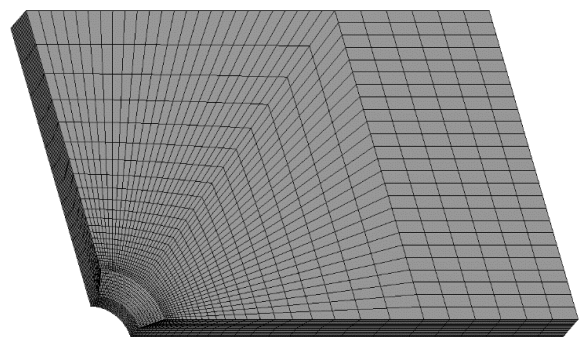


Figure 4. The final FEA mesh configuration

The results of this study, as shown in Figure 3, show that the $K_{t,\epsilon}$ value reached a converged value at mesh model 3 with a radius-to-element size ratio ($r/e=25.3$) with a relative error

of less than 0.01 per cent. For this reason, mesh model 3 which contains 25200 elements, and 28158 nodes was adopted throughout the analysis of the present work. The details of mesh model are depicted in Figure 4.

It is important to mention that this FEA model was thoroughly verified in a previous work of the author by comparing stress concentration data of this model with literature Darwish et al. [22, 23]. Therefore, this FEA model can be confidently used for further analysis of the present problem.

4. STRAIN CONCENTRATION FACTOR EQUATION

The general form of the strain concentration factor equation of the present work is chosen to be similar to that provided by Darwish et al. [22], as:

$$K_{t,\epsilon} = K_H \times K_{ss,\epsilon} \times K_{Cs,\epsilon} \times K_{\theta_c,\epsilon} \quad (2)$$

where, K_H is the S ϵ CF that includes the effect of the plate with on $K_{t,\epsilon}$. This factor considers the plane stress state. The ϵ subscript is removed here as K_H as S ϵ CF and SCF are the same in plane stress condition. The second factor, $K_{ss,\epsilon}$, carries the influence of the plate thickness as well as Poisson's ratio on $K_{t,\epsilon}$. For thicker plate configuration, the problem transforms to the plane strain state. The third ($K_{Cs,\epsilon}$) and fourth ($K_{\theta_c,\epsilon}$) factors include the effect of the countersink depth and angle on $K_{t,\epsilon}$, respectively.

For further information on the formulation and curve fitting procedures of the equations above, the reader is strongly encouraged to refer to references Darwish et al. [22, 23].

4.1 The K_H equation

For K_H formulation, the plate width effect, which is represented by radius to width ratio r/w , is assumed to be as formulated in Ref. [22], thus:

$$K_H = 3 + \frac{\left(\frac{r}{w}\right)^{1.4}}{1 - \left(\frac{r}{w}\right)^{0.5}} \quad (3)$$

The above equation accounts for the limiting cases of the plate width. Specifically, when the width is infinite ($r/w=0$) the S ϵ CF is equal to 3. Also, with a narrow plate ($r/w=1$) the S ϵ CF approaches to infinity. It is important to mentioned here that there is another applicable form for the S ϵ CF in a finite-width plate with a circular hole as presented by Heywood [35]. This form is $K_H = \frac{2 + \left(1 - \frac{r}{w}\right)^3}{1 - \frac{r}{w}}$ and it counts for the limited cases of Eq.

(3). The form of Eq. (3) was proved to be much accurate that Heywood's equation [22]. Therefore, the present paper considers Eq. (3) for the rest of the analysis.

4.2 The $K_{ss,\epsilon}$ equation

As mentioned previously, the influence of the plate thickness, which is represented by thickness to radius ratio t/r , and the Poisson's ratio ν on $K_{t,\epsilon}$ is formulated in the non-dimensional parameter $K_{ss,\epsilon}$, therefore:

$$K_{ss,\epsilon} = \left(\frac{1 - \nu^2}{1 - \nu^2 e^{-m\left(\frac{t}{r}\right)}} \right) \left(1 + \frac{a\left(\frac{t}{r}\right)}{b + \left(\frac{t}{r}\right)^c} \right) \quad (4)$$

Similar to K_H equation, the form of $K_{ss,\epsilon}$ chosen is such that it satisfies the limiting conditions of the plane problem ($K_{ss,\epsilon}=1$ as $t/r \rightarrow 0$ and $K_{ss,\epsilon}=1-\nu^2$ as $t/r \rightarrow \infty$). The values of the four constants a, b, c and m were determined through the use of non-linear regression by minimizing the error between $K_{ss,\epsilon}$ equation results and FEA data. It is important to mention here that the data used in this fit was obtained based on the straight shank problem ($C_s/t=0$) and per the equation $K_{ss,\epsilon}=K_{t,\epsilon}/K_H$ in which $K_{t,\epsilon}$ values are from FEA results and K_H data are from Eq. (3) above. Thus, the final form of $K_{ss,\epsilon}$ is:

$$K_{ss,\epsilon} = \left(\frac{1 - \nu^2}{1 - \nu^2 e^{-0.17\left(\frac{t}{r}\right)}} \right) \left(1 + \frac{0.3\left(\frac{t}{r}\right)}{5 + \left(\frac{t}{r}\right)^2} \right) \quad (5)$$

Table 2 shows the comparison between this equation with present FEA data and $K_{ss,\epsilon}$ equation developed by Bhargava and Shivakumar [34]. The comparison here showed great agreement between three solutions.

Table 2. Comparison between Eq. (5), $K_{ss,\epsilon}$ of Bhargava with present FEA results. ($\%Error = \frac{K_{ss,\epsilon} - K_{ss,\epsilon}^{FEA}}{K_{ss,\epsilon}^{FEA}} * 100\%$)

t/r	ν	$K_{ss,\epsilon}$ FEA	$K_{ss,\epsilon}$ Eq. (5)	%Error	$K_{ss,\epsilon}$ Bhargava et al. (2008)	%Error
0.5	0.2	1.014	1.025	1.116	1.024	0.978
0.5	0.3	1.016	1.020	0.414	1.018	0.197
0.5	0.4	1.019	1.013	-0.558	1.009	-0.899
1	0.2	1.019	1.043	2.358	1.032	1.275
1	0.3	1.024	1.034	0.971	1.022	-0.243
1	0.4	1.029	1.020	-0.905	1.005	-2.317
2	0.2	1.019	1.054	3.489	1.041	2.187
2	0.3	1.020	1.037	1.659	1.022	0.143
2	0.4	1.020	1.011	-0.841	0.993	-2.674
3	0.2	1.012	1.047	3.437	1.045	3.224
3	0.3	1.008	1.024	1.568	1.019	1.065
3	0.4	1.001	0.989	-1.151	0.980	-2.070
4	0.2	1.008	1.036	2.783	1.047	3.847
4	0.3	1.001	1.008	0.721	1.015	1.440
4	0.4	0.991	0.967	-2.408	0.969	-2.175

4.3 The $K_{C_s, \epsilon}$ equation

As stated earlier, the $K_{C_s, \epsilon}$ factor accounts for the influence of the countersink depth on $K_{t, \epsilon}$. This effect is included in the nondimensional geometric parameter C_s/t . The $K_{C_s, \epsilon}$ factor becomes important when the geometry of the hole transfers from the straight shank configuration to the countersunk shape. In the earlier equation of the SeCF developed by Bhargava et al. (2008) the $K_{C_s, \epsilon}$ equation was expressed in the terms of t/r and C_s/t only. However, this was proven to be not entirely true in the case of SCF, especially for high values of C_s/t and t/r . The work [22] showed that K_{C_s} is also a function of the plate width, in other words r/w . Additionally, and as mentioned previously, SeCF and SCF equations are highly similar. Therefore, the present work suggests that $K_{C_s, \epsilon}$ is to include the effect of r/w . Therefore, the general form of $K_{C_s, \epsilon}$ is:

$$K_{C_s, \epsilon} = 1 + \left(\frac{r}{w}\right)^i \left(\frac{t}{r}\right) \left(\frac{C_s}{t}\right) + d_1 \left(\frac{t}{r}\right)^j \left(\frac{C_s}{t}\right) + d_2 \left(\frac{t}{r}\right)^k \left(\frac{C_s}{t}\right)^2 \quad (6)$$

The form of $K_{C_s, \epsilon}$ above would not contribute to $K_{t, \epsilon}$ if the hole is a straight shank ($C_s/t=0$) as $K_{C_s, \epsilon}$ will be equal to 1. Additionally, for a thin plate with a central circular hole ($C_s/t=0$, $t/r \rightarrow 0$) both $K_{C_s, \epsilon}$ and $K_{ss, \epsilon}$ will be equal to 1 and have no influence on $K_{t, \epsilon}$. Also, for a wide plate ($r/w=0$), the first term in the Eq. (6) will vanish and r/w will have no effect on $K_{C_s, \epsilon}$ value.

As previously performed in the formulation of $K_{ss, \epsilon}$, the constants i, j, k, d_1 and d_2 values were determined, thus:

$$K_{C_s, \epsilon} = 1 + \left(\frac{r}{w}\right)^{1.8} \left(\frac{t}{r}\right) \left(\frac{C_s}{t}\right) + 0.28 \left(\frac{t}{r}\right)^{0.1} \left(\frac{C_s}{t}\right) + 0.1 \left(\frac{t}{r}\right)^{1.5} \left(\frac{C_s}{t}\right)^2 \quad (7)$$

In this equation, the values of $i=1.8, j=0.1$ and $k=1.5$ means that the effect of r/w and t/r in these equation terms is non-linear. Also, the values of $d_1=0.28$ and $d_2=0.1$ indicate that the effect of the interaction effect of t/r and C_s/t has a positive effect of the value of $K_{C_s, \epsilon}$. The comparison between the results of $K_{C_s, \epsilon}$ of Eq. (7), $K_{C_s, \epsilon}$ from [10], and the present FEA is depicted in Figure 5. For this figure, $C_s/t=0.25$ and $t/r=1$.

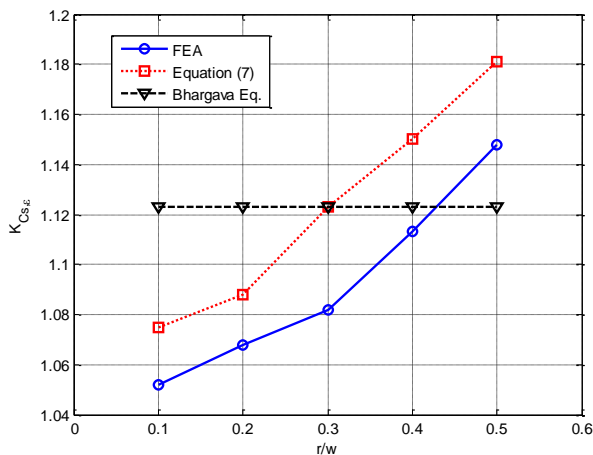


Figure 5. Comparison between Eq. (7), Bhargava's Eq. and FEA for $K_{C_s, \epsilon}$

Apparently, the $K_{C_s, \epsilon}$ obtained from the equation [34] is independent from r/w . In contradiction, Eq. (7) and FEA results proves a proportional relationship between $K_{C_s, \epsilon}$ and r/w which further justifies the selection of Eq. (6) form.

4.4 The $K_{\theta_c, \epsilon}$ equation

The general form of $K_{\theta_c, \epsilon}$ equation is selected to be similar to that developed by Bhargava and Shivakumar [34], as:

$$K_{\theta_c, \epsilon} = 1 + m(\theta_c - 100^\circ) \quad (8)$$

where, m is the slope and it is a function of C_s/t and t/r , thus:

$$m = A_1 \left(\frac{t}{r}\right)^\lambda \quad (9)$$

$$A_1 = \frac{C_s}{t} \left[-0.003 + 0.078 \left(\frac{C_s}{t}\right) - 0.078 \left(\frac{C_s}{t}\right)^2 \right] \quad (10)$$

$$\lambda = \frac{C_s}{t} \left[3.6 - 9.6 \left(\frac{C_s}{t}\right) + 7.8 \left(\frac{C_s}{t}\right)^2 \right] \quad (11)$$

In the equations above, $K_{\theta_c, \epsilon}$ equals to 1 when $\theta_c=100^\circ$ and the slope m reduces to zero for $C_s/t=0$ or for a straight shank configuration. The selection of the equation models in Eq. (8) to Eq. (12) was based on the findings [22, 34] in which the slope (m) of Eq. (8) was found to be a function of both C_s/t and t/r therefore Eqns. (9), (10) and (11) were introduced.

Although Eq. (8) to Eq. (11) are expressed as presented by [34], the coefficients of Eq. (10) and Eq. (11) are obtained based on the present FEA data and are different from those [34]. The results of the present $K_{\theta_c, \epsilon}$ equation are validated with the present FEA results and $K_{\theta_c, \epsilon}$ equation in the study [34], as presented in Figure 6. In this figure, $r/w=0.1, t/r=2$ and $C_s/t=0.3$. From this figure, the results of the present equation of $K_{\theta_c, \epsilon}$ compares well with FEA and literature.

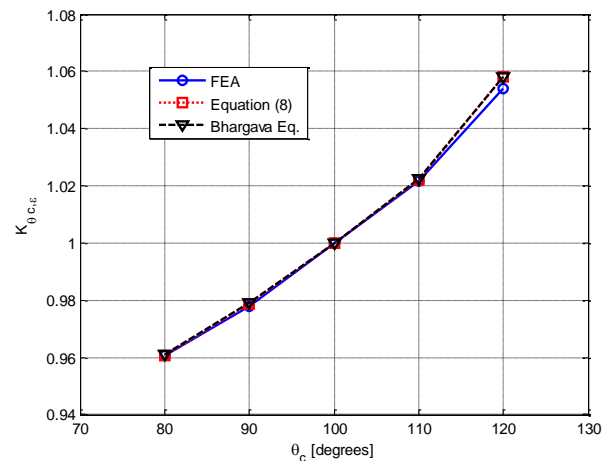


Figure 6. Comparison between Eq. (8), Bhargava's Eq. and FEA for $K_{\theta_c, \epsilon}$

5. RESULTS AND DISCUSSIONS

The effect of the dimensionless geometric parameters $r/w, t/r, C_s/t$ and θ_c as well as Poisson's ratio on $K_{t, \epsilon}$ value using the presently developed and verified equation are presented in this

section. Specifically, Figure 7 introduces the effect of the radius to width ratio (r/w). Apparently, the $K_{t,\epsilon}$ value increases as r/w increases. In other words, as the plate becomes narrower, and the plate edge gets closer to the hole radius. Besides, these three figures show that $K_{t,\epsilon}$ value gets higher as t/r , C_s/t and θ_c increase.

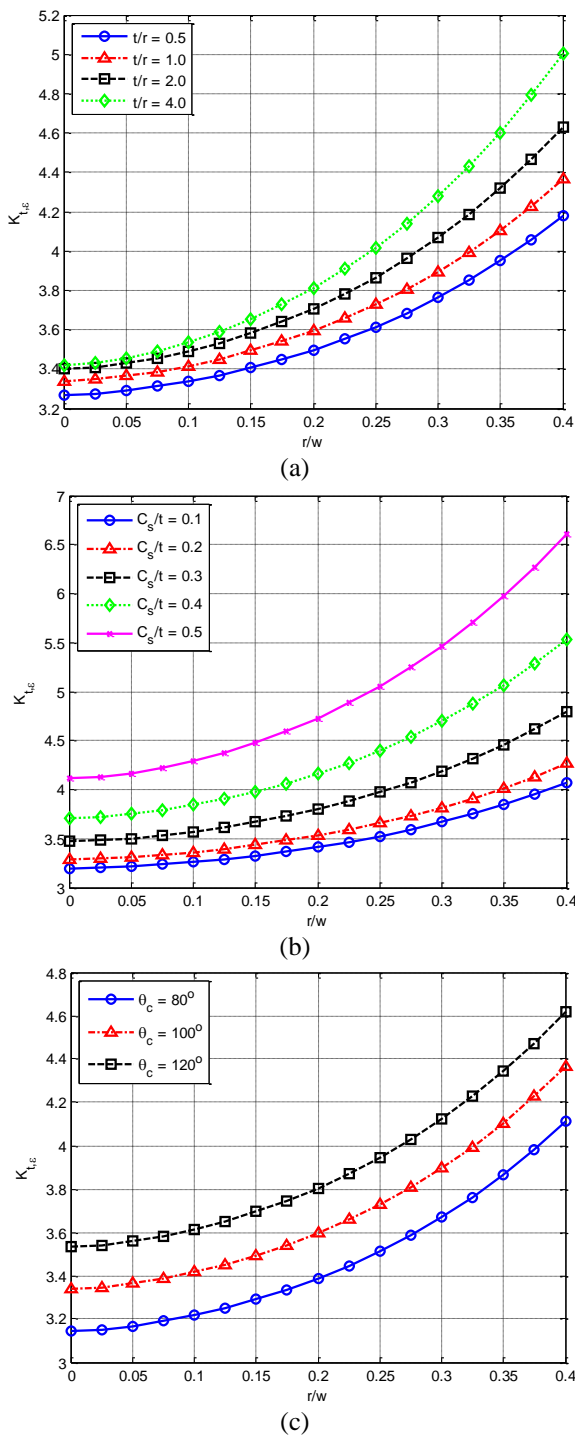


Figure 7. Effect of r/w on $K_{t,\epsilon}$ at different (a) t/r , (b) C_s/t and (c) θ_c values ($\nu=0.3$)

Similarly, Figure 8 shows the effect of t/r at different C_s/t and θ_c values. As shown in Figure 8(a), for small countersink depths ($C_s/t=0.1$), the plate thickness to radius ratio t/r effect is minor. However, for larger countersink depths ($C_s/t \geq 0.2$), t/r effect becomes more significant. Also, as presented in Figure 8(b), the plate thickness effect becomes more effective for

higher countersink angle systems. This could be explained as for higher countersink depths, the tapered surface of the hole will become thicker and therefore, extra force lines will be reoriented towards the straight shank portion of the countersunk hole.

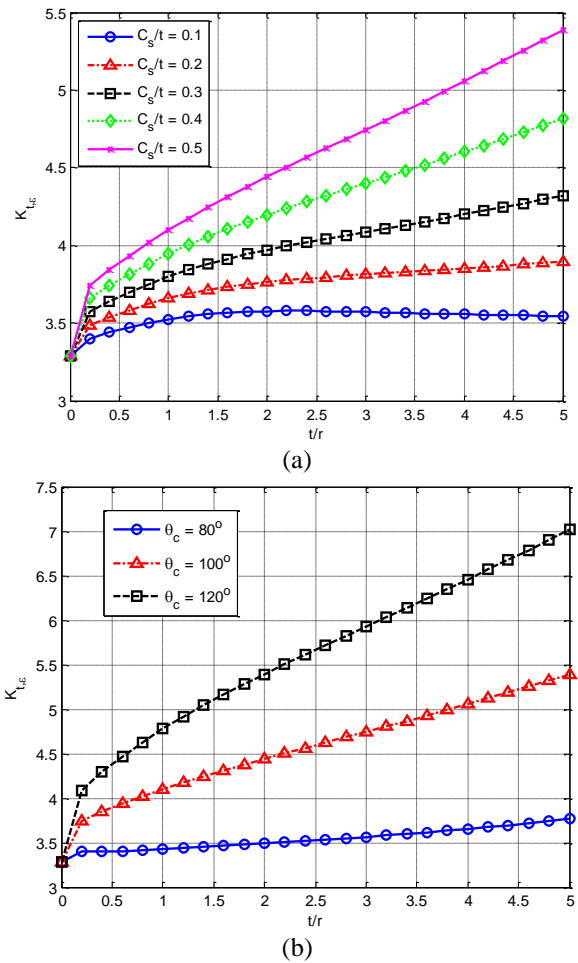


Figure 8. Effect of t/r on $K_{t,\epsilon}$ at different (a) C_s/t and (b) θ_c values ($\nu=0.3$)

Figure 9 represents the relationship between $K_{t,\epsilon}$ and C_s/t at different countersink angles. From this figure, generally, as C_s/t increases, the maximum SeCF increases, too. Additionally, this effect becomes more effective for plates with higher countersinking angles.

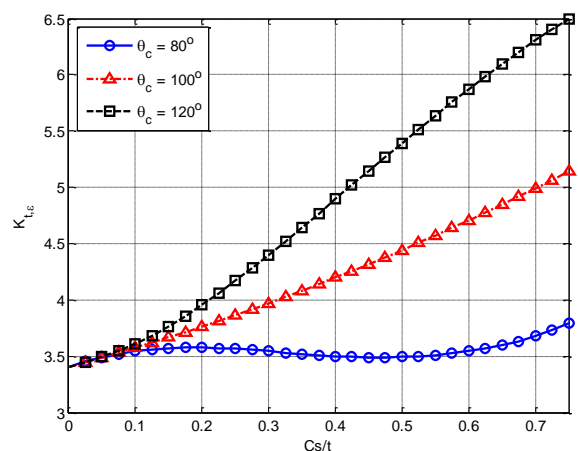


Figure 9. Effect of C_s/t on $K_{t,\epsilon}$ ($r/w=0.25$, $t/r=2$ and $\nu=0.3$)

Figure 10 depicts the relationship between θ_c and $K_{t,c}$. It is clearly seen that $K_{t,c}$ proportionally increases as θ_c increases. This is due to the fact that for larger countersink openings (larger θ_c) the hole further deviates from the straight shank configuration which results in higher values of $S\epsilon CF$.

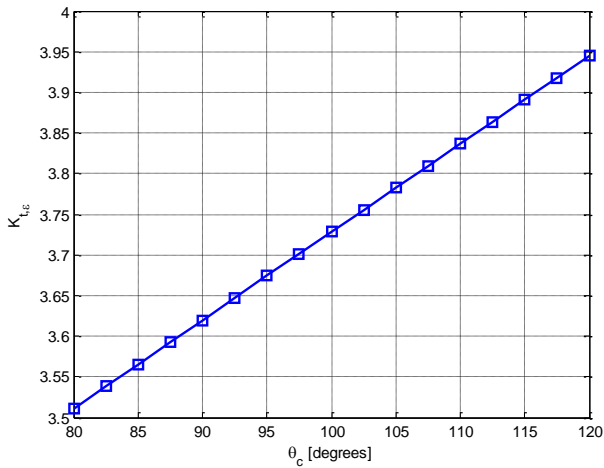


Figure 10. Effect of θ_c on $K_{t,c}$ ($r/w=0.25$, $t/r=1$, $C_s/t=0.25$ and $\nu=0.3$)

The effect of Poisson’s ratio (ν) effect on $K_{t,c}$ is presented in Figure 11. This figure shows that Poisson’s ratio effect varies from no effect, for thin plates ($t/r=0.05$), to significant effect, for thick plates ($t/r=4$). It also shows that the $K_{t,c}$ decreases as ν increases.

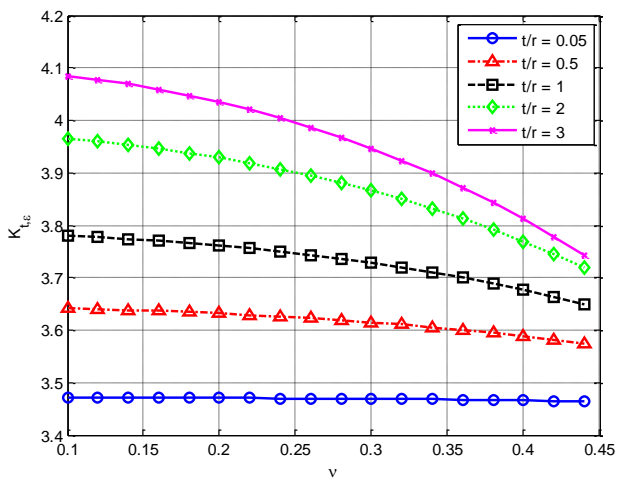


Figure 11. Effect of ν on $K_{t,c}$ ($r/w=0.25$, $C_s/t=0.25$ and $\theta_c=100^\circ$)

Finite element solutions were used to investigate the strain concentration factor distribution over the thickness of the countersunk hole. It was found that the maximum strain concentration factor value occurs at the side of the hole at 90 degrees from the load application direction. Figure 12 represents the strain concentration factor values variation over the thickness of the countersunk hole for different countersink depths and angles. From Figure 12(a), the location of the maximum strain concentration factor value changes with varying the countersink depth (C_s/t) value. On the other hand, the location of the maximum strain concentration factor value is kept fixed for different countersink angle (θ_c) values, as shown in Figure 12(b).

Appendix A lists the results of $K_{t,c}$ from the present FEA model, the newly developed equation in this study and the equation presented by Bhargava et al. for wide ranges of r/w , t/r and C_s/t at $\theta_c=100^\circ$ and $\nu=0.3$. The FEA data in table were for geometric configurations that were used in the fitting process and for new configurations that were generated for comparison and validation purposes. The relative error in $K_{t,c}$ of both empirical equations was calculated with respect to the present FEA data and appended in Appendix A as well. As it can be concluded from the error columns of this table, that for $C_s/t < 0.5$ the error values in both equations are relatively small and comparable with each other.

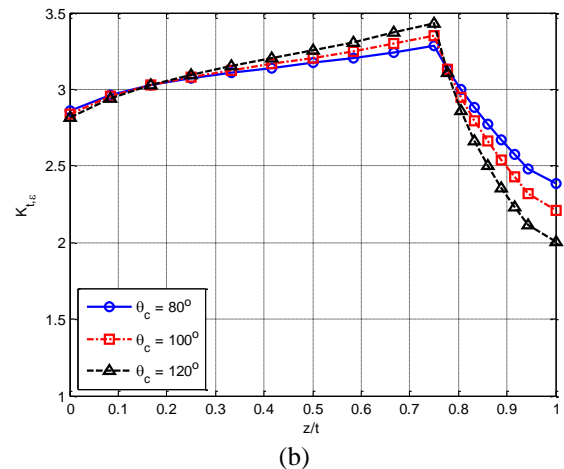
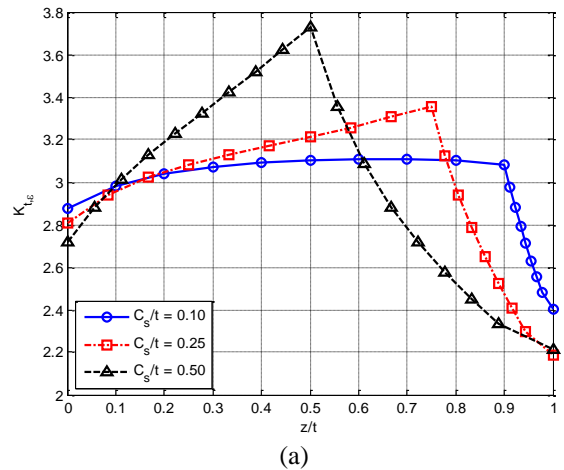


Figure 12. Strain concentration factor variation over the thickness of the hole for different countersink (a) depths, and (b) angles ($r/w=0.1$, $t/r=1$, $C_s/t=0.25$ and $\nu=0.3$)

Hence, no further significance of one equation over the other. However, for $C_s/t \geq 0.5$ the presently developed equation of $K_{t,c}$ is in better agreement with FEA data than that for Bhargava et al. equation, especially for higher r/w values and for most t/r ratios. This could be explained as the earlier equation of the $S\epsilon CF$ developed by Bhargava and Shivakumar [34], the $K_{C_s,c}$ equation was expressed in the terms of t/r and C_s/t only and did not consider the significant effect of r/w . In contrast, the present work suggests that $K_{C_s,c}$ is to include the effect of r/w of Eq. (6). Therefore, higher accuracy of the present equation is observed. Additionally, the negative error values in Bhargava et al. (2008) equation mean that this equation underestimates the $K_{t,c}$ values which could lead to erroneous results if it was used in an engineering design process. This further appraises the present $K_{t,c}$ equation over the old equation in Ref. [34].

Figure 13 shows the stress and strain contour plots of a plate with a countersunk hole with $r/w=0.1$, $t/r=2$, $C_s/t=0.5$, $\theta_c=100^\circ$ and $\nu=0.3$. This figure shows that both stress and strain concentrations are located at the countersink hole edge, as mentioned previously. Also, this figure shows that the stress and strain distribution far from the hole are equal to E and 1, respectively, which are both equal to the nominal stress and strain values. Therefore, the stress and strain concentration factors could be easily calculated as 4.1 and 3.977, respectively.

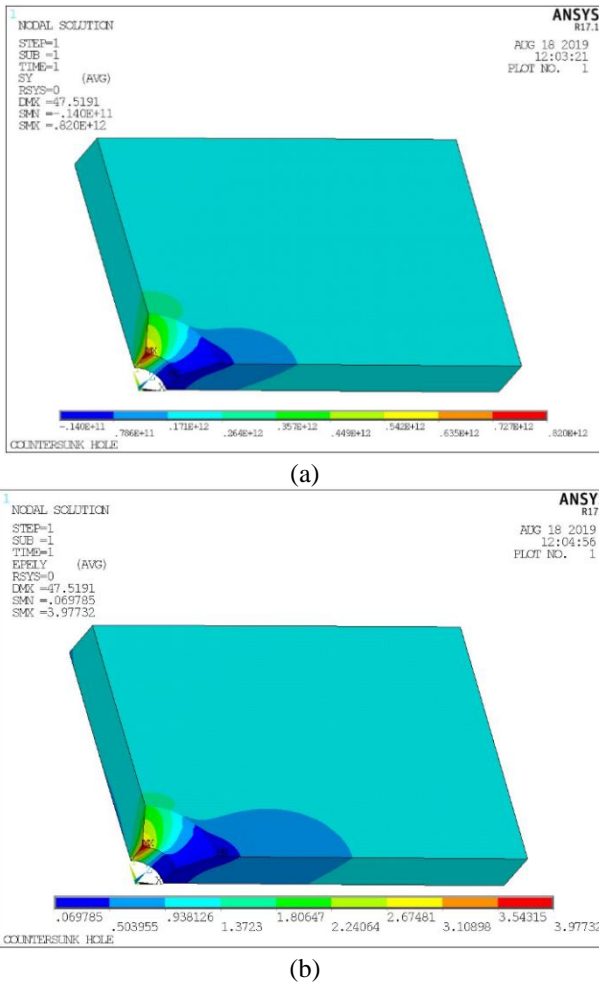


Figure 13. (a) Stress and (b) strain contour plots

6. CONCLUSIONS AND METHOD LIMITATIONS

6.1 Conclusions

This paper presented an extensive three-dimensional finite element analysis for the strain concentration in countersunk holes subjected to uniaxial tension. The influence of the four non-dimensional geometric parameters: countersink angle (θ_c), thickness to radius ratio (t/r), countersink depth to plate thickness ratio (C_s/t) and radius to width ratio (r/w) and Poisson's ratio (ν) on the strain concentration factor was investigated. Finite element analysis results showed that the maximum strain concentration is located at the countersinking edge of the hole and that is perpendicular to the load direction. Non-linear regression techniques were employed to develop an empirical equation for the strain concentration factor considering the limiting conditions and based of finite element

data. The results of the equation were thoroughly correlated with finite element analysis results. Additionally, the present equation was compared to a previously developed equation by [34] and showed better agreement with FEA results especially for countersunk holes with high radius to width ratios.

6.2 Method limitations

The presently developed equation is accurate, simple, and easy to implement. However, it includes some limitations, thus, improvements are recommended. Firstly, the FEA data that was used to fit the equation were based on the assumption that the plate deformations are restricted to the linear and elastic range only. Nonetheless, in reality stress and strain concentration lead to high deformation regions which might change the case to a nonlinear problem. Thus, the results of this equation may not be very accurate for high concentration values, i.e., larger than 4. Secondly, the present work neglects the triaxiality of the stress and/or strain. Therefore, another study, which counts for the triaxiality of stress, is essential. Such work is available in the study of Tilan et al. [36]. Lastly, the results of the current equation are highly recommended to be validated with experimental data for high-confident in the empirical formulations and findings.

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NOMENCLATURE

b straight shank thickness
 C_s countersink depth
 E, ν Modulus of Elasticity and Poisson’s ratio
 K_H dimensionless factor, introduces the effect of the plate width on $K_{t,\epsilon}$
 $K_{SS,\epsilon}$ dimensionless factor, introduces the effect of the plate thickness on $K_{t,\epsilon}$
 $K_{Cs,\epsilon}$ dimensionless factor, introduces the effect of the countersunk depth on $K_{t,\epsilon}$
 $K_{t,\epsilon}$ theoretical strain concentration factor

$K_{\theta c,\epsilon}$ dimensionless factor, introduces the effect of the countersink angle on $K_{t,\epsilon}$
 h plate half-length
 r straight shank radius
 SCF stress concentration factor
 $S_{\epsilon CF}$ strain concentration factor
 SS straight shank
 t plate thickness
 w plate half-width
 x, y, z Cartesian coordinate system
 θ_c countersink angle

APPENDIX

Appendix A. Comparison between FEA and current equation

r/w	$t/r = 1$					$C_s t = 0.1$ $t/r = 2$					$t/r = 4$				
	FEA	Eq. (2)	%E	Bhargava et al.	%E	FEA	Eq. (2)	%E	Bhargava et al.	%E	FEA	Eq. (2)	%E	Bhargava et al.	%E
0.1	3.18	3.31	4.0	3.29	3.5	3.20	3.38	5.6	3.40	6.3	3.37	3.383	0.5	3.51	4.1
0.2	3.30	3.47	5.1	3.41	3.5	3.36	3.50	4.2	3.57	4.7	3.53	3.582	1.5	3.63	2.9
0.3	3.51	3.70	5.3	3.64	3.5	3.59	3.64	1.7	3.75	4.8	3.82	3.914	2.5	3.87	1.3
0.4	3.92	4.13	5.4	4.01	2.3	3.97	4.10	2.9	4.14	4.2	4.32	4.433	2.6	4.27	-1.2
r/w	$t/r = 1$					$C_s t = 0.25$ $t/r = 2$					$t/r = 4$				
	FEA	Eq. (2)	%E	Bhargava et al.	%E	FEA	Eq. (2)	%E	Bhargava et al.	%E	FEA	Eq. (2)	%E	Bhargava et al.	%E
0.1	3.43	3.47	1.0	3.47	1.0	3.60	3.59	0.0	3.70	0.1	3.82	3.71	-3.0	3.99	4.4
0.2	3.57	3.65	2.4	3.59	0.7	3.76	3.81	1.5	3.80	1.2	4.04	4.00	-1.1	4.06	0.4
0.3	3.85	3.96	2.8	3.83	-0.5	4.09	4.18	2.2	4.05	-1.0	4.53	4.49	-0.9	4.33	-4.9
0.4	4.33	4.43	2.4	4.22	-2.5	4.68	4.76	1.8	4.47	-4.4	5.43	5.25	-4.6	4.78	-9.0
r/w	$t/r = 1$					$C_s t = 0.5$ $t/r = 2$					$t/r = 4$				
	FEA	Eq. (2)	%E	Bhargava et al.	%E	FEA	Eq. (2)	%E	Bhargava et al.	%E	FEA	Eq. (2)	%E	Bhargava et al.	%E
0.1	3.81	3.77	-1.2	3.73	-2.3	4.03	4.03	0.0	4.08	1.2	4.37	4.50	3.1	4.57	4.7
0.2	3.99	4.00	0.3	3.86	-3.3	3.36	4.34	-0.4	4.23	-3.0	4.93	4.96	0.6	4.73	-4.0
0.3	4.36	4.38	0.5	4.11	-5.8	4.87	4.86	-0.3	4.51	-7.5	-	-	-	-	-
0.4	5.03	4.97	-1.1	4.54	-9.7	5.99	5.76	-3.7	4.97	-17.0	-	-	-	-	-
r/w	$t/r = 1$					$C_s t = 0.75$ $t/r = 2$					$t/r = 4$				
	FEA	Eq. (2)	%E	Bhargava et al.	%E	FEA	Eq. (2)	%E	Bhargava et al.	%E	FEA	Eq. (2)	%E	Bhargava et al.	%E
0.1	4.05	4.10	1.4	3.94	-3.0	4.51	4.59	1.9	4.45	-1.4	-	-	-	-	-
0.2	4.25	4.38	3.1	4.08	-4.0	4.80	4.99	4.0	4.60	-4.2	-	-	-	-	-
0.3	4.70	4.84	3.1	4.35	-7.3	5.41	5.66	4.5	4.91	-9.5	-	-	-	-	-
0.4	5.48	5.56	1.4	4.80	-12.4	-	-	-	-	-	-	-	-	-	-