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# Surface Modification of Steel by Nickel Coating in Electrochemical Process

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### ABSTRACT

Due to the rebar corrosion problems, quite the value of construction is being spent towards repair and renovation of the concrete structures. If rebar corrosion problem continues, lifeline structures are subjected to sudden collapse. So as to avoid the catastrophic failures, effective additional protective measures must be undertaken within the construction stage itself. The target of this investigation is to switch the surface of the low-carbon steel rebar by electroplating in three different coating thicknesses. The efficiency of corrosion resistance properties are evaluated by various electrochemical corrosion tests. This study starts from Brenner and Riddel who developed their work by electroless deposition of nickel, the aim of this study was to calculate the corrosion rate by gravimetric weight loss method supported the electrochemical corrosion test. Also to reinforce the performance of corrosion resistance with three different coating thicknesses like  $10\mu$ ,  $20\mu$ , and  $30\mu$  supported the time consumption in low-carbon steel rebar with and without 3.5% Nacl. Potential time studies confirmed the steadiness of coating even within the presence of aggressive chloride medium during one month exposure. 12V accelerated impressed voltage test results indicate that the time taken for initial crack was doubled for  $20\mu$  coated steel.

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### **1. INTRODUCTION**

Electroless (autocatalytic) plating involves the presence of a chemical reducer in solution to scale back metallic ions to the metal state. the invention of electroless coatings is credited to Abner and Riddell [1] within the year 1940s. Today electroless nickel has grown up into a really substantial segment of the metal products finishing industry. Engineering applications for electroless nickel are often found in virtually every industry. Various physical characteristics of electroless nickel coatings, like hardness, wear resistance, coating uniformity and corrosion resistance, also because the ability to plate non-conductive surfaces make this a coating of choice for several engineering applications. The corrosion of reinforcing steel is usually determining by the effect of chemical admixtures, on the corrosion of embedded steel reinforcement in concrete, exposed to chloride environments like electrochemical corrosion methods of potential time studies and by means of the impressed current technique etc. this is often done to induce a big degree of corrosion of reinforcing bars embedded in concrete in limited available time. The protection behaviour of steel reinforcement embedded in concrete with different coatings has been frequently wont to study the corrosion performance of steel embedded in chloride contaminated concrete, effect of reinforcement corrosion on the cracking of concrete cover, bond behaviour, and load-bearing capacity of ferroconcrete structural members, potential time measurements, and impressed current technique as shown in Table 1.

The need for protective coating to low-carbon steel reinforcement bar is to develop necessary bond strength at the rebar concrete interface for the reliable performance of the ferroconcrete structures. Bond behaviour of the galvanized inhibited cement slurry coated and fusion bonded epoxy coated bars with different coating thickness is assessed and compared. It had been observed that the coated steel rebars improves the bond strength to plain low-carbon steel bars. The galvanizing and epoxy coating reduce the bond strength at higher thickness of coatings. On the opposite hand inhibited cement slurry coated rebars bond strength improves further at the upper thickness of coating. Epoxy shows that the coating thickness and therefore the damage present within the coatings will influence the adhesion loss between the steel surface and coatings and blister formation. Fusion bonded epoxy coating is one among the effective methods of coating rebars. The fusion bonded coating may be a specialized job carried during a factory and not at the location work. Although epoxy coated rebars have a superb protection to corrosion in an aggressive environment, there are a couple of limitations. The coating may get damaged during the vibration of concrete. The treatment is extremely costly nearly as costly as that of steel. All the above, this method of protection to the steel is being given to a flyover and other structures at Mumbai. The objective of the coating to steel rebar is to supply a durable barrier to aggressive materials. The coatings should be robust to face up to the fabrication of reinforcement cage, and therefore the pouring of concrete and compaction by vibrating needle. Central electro chemical research institute (CECRI) karaikudi has suggested various methods for prevention of corrosion in steel reinforcement in concrete. The steps involved during this process are

- i. Defrosting
- ii. Phosphate

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iv. Sealing **Table 1.** Review of literature studies

iii.

Cement coating

S.No	Authors	Title	Types of study	Remarks
[1]	Abner and Rieddel [1950]	Nickel plating by chemical reduction	Electroplating chemical reaction	Developed formulations and practices for Ni-P deposition on carbon steel without the aid of an electric current.
[2]	Rengasamy et al. [1988]	Inhibited and sealed cement slurry coating of steel rebar	Performance evaluation test, alternate wetting and drying test and performance in the presence of chloride admixture were also discussedelaborately	Applied to steel rebars for its corrosion protection lead to economy and efficiency higher than other coating systems.
[3]	Singh and Ghosh [2006]	Electroless nickel– phosphorus coatings to protect steel reinforcement bars from chloride induced corrosion	EIS and DC polarization techniques, SEM and XRD studies have been used to study the kinetics and mechanism of corrosion protection and to characterize corrosion products.	Medium phosphorus content is more impervious and protective in comparison to high phosphorus coating (16%). XRD studies show that the medium phosphorus electroless nickel (MEPEN) is microcrystalline in nature and forms Ni <sub>2</sub> O <sub>3</sub> and Ni <sub>5</sub> P <sub>2</sub> passive films on its surface.
[4]	Pyc et al [1998]	Corrosion Protection Performance of Corrosion Inhibitors and Epoxy- Coated Reinforcing Steel in a Simulated Concrete Pore Water Solution	Adhesion tests and hardness.	Coating thickness and damage present in the coating would influence the adhesion loss between the coatings and steel surface and blister formation.
[5]	Sidney Freedman [1998]	Corrosion Resistance of Reinforcement in Architectural precast concrete	Chloride ingress and carbonation. Criteria for evaluating crack widths, Methods of protecting reinforcement.	High quality concrete with strength between 34.50 and 41.4 Mpa and water cement ratio of 0.4 or less is adequate in preventing corrosion.
[6]	Ahmad Shamsad [2009]	Techniques for inducing accelerated corrosion of steel in concrete	The effectiveness of electrochemical techniques applied for the prevention of reinforcement corrosion	The impressed current technique commonly used for accelerating reinforcement corrosion in small- as well as large-sized concrete specimens in the light of state-of-the-art information.
[7]	El Maaddawy, and Soudki, [2003]	Effectiveness of Impressed Current Technique to simulate corrosion of steel in concrete	The influencing of varying the impressed current density level between 100 and $500\mu$ A/cm <sup>2</sup> .	The result showed that, accelerated corrosion using the impressed current technique was effective in inducing corrosion of the steel reinforcement in concrete with respect to Faraday's law.
[8]	Thangavel et al [1995]	Influence of Protective coatings on steel- concrete bond	Bond behavior of the galvanized inhibited cement slurry and fusion bonded epoxy coated bars with two different coating thickness is assessed and compared	The galvanizing and epoxy coating reduce the bond strength at higher thickness of coatings. On the other hand for inhibited cement slurry coated rebars, bond strength improves at higher thickness of coatings
[9]	Haji [2008]	Performance evaluation of protective coating on steel rebars	Chemical resistance test, applied voltage test, impressed voltage test, open circuit potential test were conducted as per IS and ASTM standards.	Chemical resistance test and applied voltage test conducted on coated specimens revealed that, coating possesses the necessary corrosion resistance properties as per standards. Impressed voltage test results indicate that the coated rebars prolongs the time required for craking appreciably as compared to uncoated specimens. Open circuit potential test revealed a low corrosion risk for all types of coated rebars
[10]	Phares et al [2006]	Evaluation of Corrosion Resistance of Different Steel Reinforcement Types	Evaluation test, Adhesion test.	From the evaluation test, to measure the difference in corrosion resistance between MMFX composite steel. Epoxy coated reinforcement and uncoated reinforcement in bridge decks to determine whether MMFX steel provides corrosion resistance superior to epoxy coated steel and to determine the inhibition of corrosion and the rate of corrosion growth in these three reinforcement materials.
[11]	ASTM Standards G109 [1999]	Standard Test method for determining the effect of chemical admixtures on the corrosion of embedded steel reinforcement in		

		concrete exposed to chloride environments.		
[12]	Lee et al. [2009]	Corrosion performance of Electroless Nickel-Plated Steel	Bond strength test	The electroplating has no significant impact on steel bond strength. In most cases, Electroless nickel-plated film showed a reduced corrosion probability.
[13]	IS- 13620: [1993]	Fusion Bonded Epoxy Coated Reinforcing Bars	Surface Preparation, and Application of coating	
[14]	ASTM A775/A775M- [1993]	Standard specification for epoxy-coated reinforcing bars, American Society for Testing and Materials.		
[15]	ASTM C-876- [1999].	Standard Test Method for Half-Cell Potentials of Uncoated Reinforcing Steel in Concrete	Open Circuit Potential Test	

## 2. MATERIAL PROPERTIES

#### 2.1 Selection and Coating for steel

The metallurgists and electrochemical engineers have developed several methods for cover of steel from corrosion in concrete. the foremost commonly known systems of protection of steel from corrosion in concrete are use of corrosion resistance steel reinforcement bars. Methods developed by Central Electro-Chemical Research Institute (CECRI), as Inhibited Cement Slurry Coating System (Old Method), and Cement-Polymer-Composite Coating System (New Method), Fusion Bonded Epoxy Coated Rebars (FBECR), Hot dip – Galvanizing, Epoxy – Phenolic Interpenetrating Polymer Network (IPN) Coating for reinforcing steel, Truncated inhibited slurry cement base coating.

Electroless nickel plating may be a chemical change which reduces nickel ions in solution to nickel metal by chemical reduction. the foremost common reducer used is sodium hypophosphite. It's estimated that sodium hypophosphite is employed in additional than 99% of all electroless nickel plating and this publication refers only to the utilization of this reducer thanks to rebar corrosion problem, effective additional protective measures must be undertaken within the construction stage itself. the target of this investigation is to switch the surface of the bare steel by electroless nickel coating in three different thickness (10 $\mu$ , 20 $\mu$ , and 30 $\mu$ ) supported the time (half an hour, 1 hour, and 1&1/2 hour). The efficiency of corrosion resistance properties of 10µ, 20µ, and 30µ coated steel were evaluated by conducting circuit potential test, tolerable limit of chloride test, accelerated impressed voltage test and various electrochemical corrosion tests. Methodology for the preparation of Nickel coated steel Briefly following the steps which are involved within the coating process are:

a. Derusting by dipping the rebars in pickling solution (patent no.465/CAL/75) for half-hour (pH of the answer is 1.04).

b. Removal from acid tank and dipping in alkaline tank to neutralize and cleaning with potable water for 10 minutes.

c. Cleaning the rod surface with water and dry it for twenty-four hours. The appliance of the coating shall not be unduly delayed and shall commence immediately after completion of surface preparation.

d. Application of first coat of nickel coating 10µ for

half-hour and drying for twenty-four hours.

e. Application of second coat of nickel coating 20µ for 45 minutes and drying for twenty-four hours.

f. Application of third coat of nickel coating 30µ for hour and drying for twenty-four hours.

## 2.2 Materials used

The cement used was Ordinary hydraulic cement (53 Grade) conforming to IS 298-1976. The compositions of OPC used are given as in Table 2.

**Table 2. Compositions of OPC** 

Constituents	Quantity
Loss on ignition	2.60%
Sio <sub>2</sub>	22.14%
Fe <sub>2</sub> o <sub>3</sub>	3.35%
Al <sub>203</sub>	11.85%
Cao	60.68%
$M_{go}$	1.30%

Locally available well-graded quarry dust, conforming to Zone-II of IS 383-1970 having relative density 2.68 and fineness modulus 2.70 was used as fine aggregate. Natural granite aggregate having density of 2700kg/m3, relative density 2.7 and fineness modulus 4.33 was used as coarse aggregate. High yield strength deformed 12mm diameter bars of Fe 415 grade was used.

### **3. EXPERIMENTAL INVESTIGATIONS**

Before conducting the test for the evaluation of protective coatings in steel reinforcement rods, the subsequent criteria are to be taken into accounts:

- a. Protective coating should have excellent corrosion resisting properties.
- b. Coating shall not affect the bond strength of steel to concrete.
- c. Coating shall withstand the handling stresses at site.
- d. Aggressiveness of test should meet the relevant ASTM/BIS standards.

The detail of test conducted inorder to guage the performance of coating systems are:

- i. Circuit Potential Test
- ii. Applied Voltage Test
- iii. Accelerated Impressed Voltage Test

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#### **3.1 Procedure for circuit Potential test**

The experimental investigations are administered at Structural Engineering Laboratory, Anna University, Chennai, Circuit potential test was conducted to guage the performance of coated rebars in nullifying and intrusive the ingress of chloride ions under alternate wetting and drying conditions. This is often a qualitative method of assessing the corrosion condition of the rebar. The potential measurements were administered as per ASTM C876-1991 standard [15]. The tendency of any metal to react with its environments indicated by the potential it develops on contact with the environment. In reinforced structures, concrete act as an electrolyte and therefore the reinforcement will develop a possible counting on the concrete environment, which can vary from place to put. The principle involved during this technique is actually measurement of corrosion potential of rebar with reference to a typical reference electrode, like saturated calomel electrode, copper/copper sulphate electrode, silver/silver electrode etc. due to its good stability, calomel electrodes are widely wont to measure the potential of embedded steel in concrete structures. The steel rebar in concrete structures should be accessible during a few locations for giving electrical connections. Figure 1 shows the circuit potential test ongoing.

The positive terminal of high impedance voltmeter is connected to exposed rebar and negative terminal to reference half-cell. The reference electrode is dipped into the answer and therefore the corresponding potentials are recorded. The observed potential readings are compared with ASTM criteria for corrosion of steel in concrete just in case of uncoated bars, and three thickness coated bars. These are referred as either circuit potential or corrosion potential.



Figure 1. Open Circuit Potential Test in progress

### 3.2 Procedure for Applied Voltage

The effects of electrical and electrochemical stress on the bond of coating to steel and film integrity of the coating were assessed by conducting applied voltage test. This is often an accelerated sort of corrosion test administered as per Indian Standard Code (IS13620 -1993). Applied voltage test is an accelerated test to guage the corrosion resistance of coated rebars in 7% NaCl as per ASTM standars. It's a top quality control test for fusion bonded epoxy coated rebar. As per the quality specification, during the primary jiffy there shouldn't be any rust spot on the coated rebar and no hydrogen evolution at the cathode rebar. The corrosion rate for uncoated and coated rebar has been evaluated and compared. The experimental investigations are administered at Structural Engineering Laboratory, Anna University, Chennai.

A non conductive plastic container of size not but 150x150x850mm was used for the test. The container was crammed with 7% NaCl solution up to a height of 800mm. The coated rebars were suspended vertically soon have a clearance of 25mm from rock bottom, 40mm from the edges and 40mm in between the rods. One reinforcement bar was connected as anode with the terminal of D.C power source, other reinforcement bar was connected as cathode. A possible of 2V was impressed in-between the 2 electrodes up to a period of hour using suitable D.C power supply. The coating failure as evidenced by the evolution of hydrogen gas at the cathode or the looks of corrosion products of iron at the anode was monitored during the trial period. A saturated calomel electrode with saturated K-Dur 20 as electrolyte and salt bridge made from agar-agar was used for measuring reference electrode potential as shown in Figure 2.



Figure 2. Applied voltage test in progress

The potential measurements were observed at every 5 minutes interval during the trial period using high impedance multitmeter. The present development was also monitored at regular intervals. During the hour of testing no coating failure was observed by evolution of hydrogen gas at the cathode or by the looks of corrosion product at the anode surface.

#### 3.3 Procedure for Impressed Voltage Test

Impressed voltage test is an accelerated corrosion test administered to assess the performance of coated rebars under accelerated electrochemical corrosion conditions. The test was administered as per the procedure prescribed by the structural engineering research centre (SERC), Chennai, and Central Electrochemical Research Institude (CECRI), Karaikudi, India, followed by the specifications of Indian Standard Code (IS13620-1993) [13] and American Standard Code (ASTM A775/ A775M-95) [14]. The test specimen comprises of 75mm diameter and 100mm height concrete cylinder, 12mm diameter and 75mm length coated rebars were centrally embedded such it gives a transparent cover of 25mm from all the edges. The coated rebar act as working electrode and therefore the top surface was soldered with 14mm gauge wire. Both top and bottom surfaces were sealed with epoxy material. the highest and bottom surfaces of the concrete specimens were also sealed with epoxy coating so on allow chloride ingress from the edges to duplicate site conditions. The chloride ions migrate through the concrete and to induce corrosion in an accelerated manner. The time taken for chloride ions to migrate depends on the entire resistance of concrete and therefore the amount of corrosion will depend upon the induced current. Figure 3 shows the test found out ongoing.

The variation of current was recorded with time. a pointy rise in current indicates the onset of corrosion. The time taken for initial crack is taken into account as a measure of the relative against chloride permeability and reinforcement corrosion. Typical current time relationship for ferroconcrete specimens tested under an applied positive potential of 12V Vs SCE.



Figure 3. Impressed voltage tests in progress

#### **3.4 Metal Loss Determination**

In the metal loss determination technique the load loss of reinforcing steel thanks to corrosion is employed to assess the magnitude of concrete deterioration. The reinforcing steel is faraway from the concrete, cleaned then weighed to calculate the metal loss. The metal loss data are then converted into corrosion rate expressed in mmpy (Milli Meter per Year) units.

#### 3.5 Determination of free chloride contents

After testing core samples were collected. The core samples (inclusive of fine and coarse aggregates) collected from 75mm depth and 12mm dia cylindrical cubes also because the cement grout were crushed mechanically and powdered samples were made. Then 50gm of powdered sample was shaken with 100ml of water during a conical flask employing a microid flask shaker for one hour. The extract was then filtered through a whatsman paper no 42. The extract prepared from the powdered samples was then analyzed for free of charge chloride content. 20cc of filter solution was taken and free chloride was estimated by standard nitrate titration using potassium chromate as an indicator.

## 3.6 Bending test

The bending test is employed to guage the effective adhesion of the coated substance on the rebar surface. The effective bond adhesion should be formed on the rebar surface and it's essential for reliable future performance of the coated steel rebar in concrete structures. The effective bond adhesion only increases the corrosion resistance property of the steel rebar and may be maintained the right activation round the steel even under the presence of tolerable limits of aggressive ions.

CTD rebars of size 60mm long and 12mm dia were defrosted during a standard pickling acid dried, and degreased with trichloroethylene. The electroless Ni plating was applied on the pickled surface of the rebars. The varied coating thickness of 10µ, 20µ, and 30µ was applied on the defrosted rebars. After a selected period, the coated rebars were subjected to guage the adhesion by means of bending test. Coated rebars were positioned over the bar bending table such two longitudinal protrustations are placed during a plane perpendicular to the mandrel radius and rebar is kept at a equilibrium of 24±20c. 10mm length of the coated rebar was fixed with the bending guide element. Then the opposite end of the coated rebar was fixed with the bending lever and applied the force to bend at 90oc. The experiment was simultaneously repeated for the opposite specimens. The surface was observed to seek out out any peal of the coated element within the corresponding element respectively.

## 3.7 Scanning microscope

The coatings are analyzed by scanning microscope (SEM) and energy-dispersive x-ray spectroscope (EDX), which unravel the coatings localized morphology and elemental distributions at the microscopic level respectively. Scanning microscope (SEM, JOEL model, JSM make, Model 6380LA) is employed to watch the surface morphology of EN coating and the basic variations within the coating low-carbon steel bar is estimated by Energy Dispersive Xray analysis (EDX) which is attached with SEM. The EDX data are then obtained with a micro-analytical unit that featured the power to detect the tiny variations of chemical element content. EDX attachment is completed in conjunction with SEM for the study of the chemical analysis and therefore the composition of the deposit in terms of the share of Nickel, Sulphur, and Carbon within the coating.

#### 4. RESULTS AND DISCUSSIONS

#### 4.1 Circuit potential test

**Table 3.** Guidelines for Probability of corrosion Based onPotential as per ASTM C876-1991

Potential, mV versus SCE	Potential, mV Vs Copper/ Copper sulphate	Probability of corrosion (%)
More –ve than -475	More –ve than -500	Severe corrosion
More –ve than -275	More –ve than -350	High risk of corrosion) (>90%)
Between -275 to -125	Between -350 & -200	Intermediate corrosion risk (Uncertain)
More +ve than -125	More +ve than -200	Low corrosion risk (<5%)

Open circuit potential (OCP) measurement readings were recorded to verify the surface areas of the corroded steel and to match the corrosion potentials of steel embedded in various specimens. The potential values were measured altogether days. Consistent with ASTM when OCP is above -125mV, there's but 5% probability that reinforcement may corrode. If the potential is between -125mV to -275mV saturated calomel electrodes (SCE), corrosion probability is uncertain. Corrosion probability could also be above -275mV to -475mV (SCE). As per ASTM C876-1991 [15] standards the probability of reinforcement corrosion is given within the Table 3 and therefore the observations at the top of exposure period as shown in Table 4. At the top of trial period, the condition of rebar inside the answer was examined to seek out the extension of corrosion.

 
 Table 4. Observation on uncoated and coated steel bars at the end of exposure period in potential test

S.No	Type of coating	Visual examination of rebar from broken specimen	Interpretation
1	Mild steel bar	Localised patchy rusted area. Few visible corrosion pits	Severe corrosion risk
2	Coated steel (10µ)	Localised patchy rusted area. No corrosion pits. Few rust spots	High (90% corrosion risk)
3	Coated steel (20µ)	No rust spots	Low corrosion risk
4	Coated steel (30µ)	No rust spots	Low corrosion risk

They are preferred frequently on rebar concrete structures because they identify the zones with different corrosion probability. The overall test method is described within the ASTM C876-1991 standard. This standard states that the potentials above -200mV versus Copper/Copper Sulphate Electrode (CSE) the probability of corrosion is extremely less (less than 5%) while below -350mV, corrosion is extremely portable (greater than 95%). Bare low-carbon steel showed active potentials throughout the trial period of 30 days. A minimum potential of -332 mV and a maximum potential of -778 mV were measured in cement extract without NaCl and therefore the other hand a minimum potential of -482mV and a maximum potential of -760mV were measured in cement extract with 3.5%NaCl as shown in Figure 4.



Figure 4. Potential Vs Time studies for bare steel rebars in cement extract

 $10\mu$  coated steel showed very low passive potentials up to 10 days of exposure. A minimum potential of -249 mV and a maximum potential of -518 mV were measured in cement extract without NaCl and the other hand a minimum potential of -454mV and a maximum potential of -603mV were measured in cement extract with 3.5% NaCl as shown in Figure 5.  $20\mu$  and  $30\mu$  Coated steel almost showed passive potentials in the range of -422mV to -557mV in the absence of NaCl and the other hand the passive potential shows potentials in the range of -410mV to -633mV in the presence of NaCl are shown in Figure 6 and 7. This is due to the formation of Ni (OH)<sub>2</sub>, stable potentials were measured.







Figure 6. Potential Vs Time studies for 20µ coated steel rebars in cement extract



Figure 7. Potential Vs Time studies for 30µ coated steel rebars in cement extract

#### **4.2 Experimental impedance time setup**

The polarization resistance  $r_p K\Omega$  values calculated for coated steel with 10 $\mu$ , 20 $\mu$ , and 30 $\mu$  thicknesses in pure cement extract as shown in Figure 8 and cement extract with 3.5% NaCl medium as shown in Figure 9, the  $r_p$  values recorded initially in pure cement extract were found to be 21.56, 23.49, and 19.43 K $\Omega$  for 10 $\mu$ , 20 $\mu$ , and 30 $\mu$  thicknesses respectively. These data indicated that in aggressive chloride medium the  $r_p$  values considerably decreased. After one month of exposure in pure cement extract the  $r_p$  values actually doubled in 53.96, 63.76, and 46.93 K $\Omega$  for 10  $\mu$ , 20 $\mu$ , and 30 $\mu$ thickness. These data indicated that in pure cement extract nickel reacts with Ca (OH)<sub>2</sub> present in cement extract and form Ni (OH)<sub>2</sub>. This stable compound considerably increased the polarization resistance. In case of cement extract + 3.5% NaCl medium the  $r_p$  values showed an increasing trend. This interesting behavior was observed due to the stability of Ni (OH)<sub>2</sub> even in the presence of aggressive chloride medium.



Figure 8. Polarization resistance Rp values (k  $\Omega$ ) in pure cement extract



Figure 9. Polarization resistance rp values (k  $\Omega$ ) in pure cement extract with 3.5% NaCl

#### 4.3 Gravimetric Weight loss measurement

Initially weighed the bare mild steel and coated steel with  $10\mu$ ,  $20\mu$ , and  $30\mu$  thickness was exposed to cement extract for 30 days of exposure. After, the specimens were taken out, cleaned in, dried and weighed. From the weight loss, corrosion rate was calculated using the formula,

Corrosion rate= (87.6 x Weight loss in mg) (Area in cm<sup>2</sup>x time in hours x density in g/cc)

The corrosion rate of bare mild steel and coated steel after 30 days of exposure in cement extract are shown in Figure 10. In the absence of Nacl, bare mild steel showed the corrosion

rate of 0.03902mmpy and the coated steel specimen showed slight decrease in corrosion rate. The corrosion rate values observed for the corresponding  $10\mu$ ,  $20\mu$ , and  $30\mu$  coating thickness were found to be 0.03852, 0.03529, 0.03524 mmpy respectively. These data clearly showed that increase in coating thickness decreased the corrosion rate markedly. On the other hand in the presence of 3.5% Nacl, the corrosion rate of bare steel 0.03759mmpy and the coated steel specimen showed slight decrease in corrosion rate. The corrosion rate values observed for the corresponding  $10\mu$ ,  $20\mu$ , and  $30\mu$  coating thickness were found to be 0.03616, 0.03600, 0.03542 mmpy respectively. These data also clearly showed that increase in coating thickness decreased the corrosion rate markedly.



Figure 10. Weight loss measurement studies for coated steel rebars in cement extract

#### 4.4 Applied voltage test

At the end of exposure period, the observation of hydrogen gas evolution for coated and uncoated steel rebar as shown in Table 5.

 Table 5. Observations at the end of exposure period for

 coated and uncoated steel rebars in applied voltage test

Type of coating (Electroless nickel coating)	Anode (At the end of one hour)	Cathode (During the test period)
Bare Mild steel	Innumerable rust spots	Vigorous evolution of hydrogen gas
10µ coated steel	No corrosion products of iron	No evolution of hydrogen gas
20µ coated steel	No corrosion products of iron	No evolution of hydrogen gas
30µ coated steel	No corrosion products of iron	No evolution of hydrogen gas

4.4.1 Gravimetric Weight loss measurement for Applied Voltage Test:

Initially weighed the bare steel and coated steel with  $10\mu$ ,  $20\mu$ , and  $30\mu$  thickness for 60 minutes of exposure. After, the specimens were taken out, cleaned in, dried and weighed. From the weight loss, corrosion rate was calculated using the formula, Corrosion rate= (87.6 x Weight loss in mg)/ (Area in cm<sup>2</sup>x time in hours x density in g/cc). The corrosion rate of bare mild steel and  $10\mu$ ,  $20\mu$ , and  $30\mu$  coated steel after 60 minutes of exposure in 7% NaCl are shown in Figure 11. The

bare mild steel showed the corrosion rate of 0.23.0667mmpy and the coated steel specimen showed 3 times decrease in corrosion rate. The corrosion rate values observed for  $10\mu$ ,  $20\mu$ , and  $30\mu$  coating thickness were found to be 8.6449, 8.5852, 8.0128mmpy respectively. These data clearly showed that increase in coating thickness decreased the corrosion rate markedly. From the above result, it was concluded as the corrosion rate decreased to 3 times when compared to bare mild steel.



Figure 11. Weight loss measurements in Applied Voltage test

### 4.5 Impressed voltage test

4.5.1 Time taken for initial crack

The time taken for initial crack for bare mild steel was observed at 23.45 hours.  $10\mu$  and  $20\mu$  coated steel showed higher values of time taken for initial crack. For example  $10\mu$ ,  $20\mu$  coated steel showed 28.45 hours and 47.30 hours as time taken for initial crack. These data indicated that  $20\mu$  coated steel actually doubled the time taken for initial crack was found to be 25.00 hours. Maximum current flow data showed 60mA for bare mild steel. On the other hand the maximum current flow considerably decreased for coated steel. For example  $10\mu$ ,  $20\mu$ ,  $30\mu$  coated steel showed 53mA, 45mA, 47mA respectively. These data showed that increasing the coating thickness from  $10\mu$  to  $30\mu$  decreased the magnitude of current flow. The amount of chloride present was expressed in terms of ppm (Parts per Million) on the basis of weight of sample taken for analysis as shown in Figure 12.



Figure 12. Amount of Chloride present in the controlled concrete

• Time taken for initial crack was doubled for  $20\mu$  coated steel.

• The magnitude of the current flow was also found to be the least at  $20\mu$  coated steel.

• For  $20\mu$  coated steel, 2 times decrease in corrosion rate was observed when compared to bare mild steel.

# 4.6 Weight loss measurements

Corrosion rate of bare steel and coated steel embedded in cylindrical concrete revealed that bare mild steel showed 0.09670 mmpy. On the other hand  $10\mu$ ,  $20\mu$  coated steel considerably decreased the corrosion rate of coated steel. For example  $10\mu$ ,  $20\mu$  coated steel showed 0.09207 mmpy, and 0.04519 mmpy respectively. These data indicated that  $20\mu$  coated steel showed a very high corrosion rate of 0.09987 mmpy are shown in Figure 13.



Figure 13. Weight loss measurement in Impressed voltage test

### 4.7 SEM test

SEM test is used to study the surface texture of the specimen [16-29]. To explore the microstructure, composition and the performance of  $10\mu$ ,  $20\mu$ , and  $30\mu$  coatings with relatively uniform top surface are selected to deposit EN coatings in mild steel reinforcement bar. These selected coatings are then analysed by SEM and EDX, which unravel the coatings, localized morphology and elemental distributions are shown in Table 6 at the microscopic level respectively. The properties of the EN depositions are attributed to their microstructure characteristics. The details of the structures of EN deposits are not clear but it has been reported to be either crystalline. amorphous, or both. By Studying the microstructure of the deposits helps us to understand the mechanism of deposition and also to evaluate the properties of deposition. The surface morphology of the coating is observed by SEM and their coated surface of 10, 20µ, 30µ is shown in Figure 14-16.

Table 6. Elemental D	istributions
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S.No	System	Element	Weight %	Atom %
		С	0.22	1.04
1	10µ	S	0.36	0.64
		Ni	99.43	98.32
		O,Cl	2.52,0.34	8.59,0.52
2	20µ	S	0.78	1.33
		Ni	96.36	89.56

		С	0	0
3	30µ	S	0.43	0.79
	-	Ni	99.57	99.21



Figure 14. SEM microphotograph of surface of EN coating in  $10\mu$  after exposure of test electrolyte



Figure 15. SEM microphotograph of surface of EN coating in 20μ after exposure of test electrolyte





Figure 14 shows the patches slightly deposit on the coated material and the grains formed above the surface. In Figure 15, it can be seen that some scratches and residual contaminants are remained on the 20µ coated steel. These defects are from the polishing process are typical and may provide localized sites for corrosion to initiate in natural environment. The good Ni coating featured very dense and homogeneous surface microstructure whereas the poor Ni coating exhibited dense yet heterogeneous microstructure. In contrast, the worst Ni coating exhibited very loose deposition of particles and the presence of contaminants in the deposited surface layer. On the other hand, the electroless deposition of Ni coatings in 10µ and 30 µ was successful, since the surface layer was dominated by the Ni element. The EDX was performed to study the composition of the coating. The EDX analysis shows that the electroless coating contains 98.32% Ni, 0.64% S and 1.04% C in 10 $\mu$ , whereas 89.56%Ni, 1.33% S, 8.59%O and 0.52%Cl are deposit in 20 $\mu$  and 99.21%Ni and 0.79%S are present in 30 $\mu$ . The EDX spectra of the coating surface are shown in Figure 17.



**Figure 17.** Shows the EDX of EN coating in (a) 10μ; (b) 20μ; (c) 30μ coated mild steel reinforcement bar

# 5. CONCLUSIONS

In this research part, the experimental study was conducted to investigate the behaviour of the nickel coated steel rebars as:

• The potential time studies carried out for coated steel in cement extract showed stable potentials in the range of -400mV to 500mV Vs SCE (-160 to -260mV Vs SHE). In presence of aggressive chloride 3.5% NaCl medium, also the coated steel showed stable potentials in the range of -450mV to -550mV Vs SCE (-210mV to -410mV Vs SHE). These data confirmed the stability of the coating even in the presence of the aggressive chloride medium during one, month exposure.

• The electrochemical impedance spectroscopy studies carried out for coated steel in cement extract showed that the Rp values increased 2 times compared to the critical values. In presence of aggressive chloride medium also, the Rp values showed an increasing trend due to the stability of the Ni(OH)<sub>2</sub> passive film.

• The corrosion rate values calculated from gravimetric method revealed heat even in the presence and absence of chloride only a slight variation in common rate was observed due to the formation of stable nickel compounds at the surface.

• 2V accelerated applied voltage test results showed a 3 times decrease in corrosion rate for coated steel when compared to bare mild steel.

• 12V accelerated impressed voltage test results indicates that

 $\circ$  Time taken for initial crack was doubled for 20  $\mu$  coated steel.

 $\circ$  The magnitude of the current flow was also found to be the least at 20µ coated steel.

 $\circ$  For 20  $\mu$  coated steel, 2 times decrease in corrosion rate was observed when compared to bare mild steel.

• The coated bar did not fail during one hour applied voltage test and passes the requirements on ASTM standards.

• Impressed voltage test results showed an increase in cracking resistance time for coated bars of the order of 3 times as compared to uncoated bars in control irrespective of the type of bar.

• The coating damage did not contribute any appreciable influence on the cracking resistance behaviour for both control and uncontrolled concrete.

• 12mm coated steel was bending at  $180^{\circ}$  with mandrel diameter 280mm for 45 seconds.  $10\mu$ ,  $20\mu$ ,  $30\mu$  coated steel showed no cracking of coating.

• The mild steel rebars coated by the Ni featured the highest noise resistance and lowest corrosion rate over the one month exposure, followed by the mild steel rebars coated by the Ni.

• The noise data confirm that the Ni coatings greatly enhanced the corrosion resistance of the steel substrate.

• This study lays the groundwork for developing a costeffective method to protect steel bar for concrete as well as industrial applications.

• The electrochemical experiments proved that the electroless Ni coatings significantly improved the steel substrate and led to corrosion behaviour.

• The coatings on the rebar surface also greatly reduced the risk of pitting corrosion and crevice corrosion in the

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