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Numerical Investigation of Concrete Slabs with Openings Reinforced by Glass Fiber-Reinforced Polymer and Strengthened with Carbon Fiber Reinforced Polymer Sheets



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GFRP reinforcement, slab opening, opening strengthening, steel plate retrofitting, CFRP sheets, finite element simulation

ABSTRACT

The existence of openings in slabs is often unavoidable and necessary for architectural reasons. These openings can compromise load-carrying capacity, energy dissipation, stiffness, and ductility of the slab. Therefore, strengthening around the opening is necessary to overcome this deficiency. On the other hand, nowadays glass fiberreinforced polymer (GFRP) rebars are widely used in concrete slabs as it can provide higher strength, lower weight, greater ductility, and non-corrosive characteristics. However, the behavior and enhancement of GFRP-reinforced concrete slab with opening has not yet been extensively investigated. Therefore, in this work, the effect of opening shape, number, and location were studied. Furthermore, the enhancement of the slab through using additional reinforcement bars and steel plates around the opening was assessed by finite elements simulation. First, the numerical models were validated by comparing their behavior with experimental tests found in the literature. After that, a parametric study was carried out to accomplish the objectives outlined in this paper. The results indicated that the opening shape has no large influence on the current slab while changing the opening location from mid-span to a location near support showed a 19.8% higher load-carrying capacity. When the slab was strengthened by a steel plate all around the opening, an increase of 40% was achieved. Likewise, by attaching CFRP sheets and strengthening with a steel plate on top of the slab, the strength was increased up to 39.2% compared to the slab strengthened with CFRP sheets only.

1. INTRODUCTION

Slabs are important structural elements carrying loads, perpendicular to their planes and directing them to columns and beams as well as horizontal loads, such as those from earthquakes. However, openings in slabs are required for various reasons. These openings are unavoidable and necessary for architecture reasons, elevator shafts, or staircases [1]. The existence of opening in slab will reduce the weight of slab and since the weight of floor can be reduced, the load on beams is diminished. Additionally, the load on support will be eased and the performance under both regular and seismic load conditions will be improved [2]. Nonetheless, the presence of openings affects the behaviour of floor diaphragms and make it unpredictable and more complicated. These openings can compromise load-carrying capacity, stiffness, energy dissipation capacity, and ductility of the slab. Designers consider additional reinforcement for these openings, but the prediction of their location is not easy in design stage due to various reasons such as heating and cooling ducts, air conditioning system placement, placement of electric, or other infrastructure. In other words, the additional reinforcement cannot be performed in the beginning. Therefore, strengthening around the opening is necessary after construction to ensure structure integrity [3]. The externally bonded technique (EBT) using glass fiberreinforced polymer (CFRP) is approved to be an effective strengthening method because of its easy installation [4-10].

Muhammed [11] carried out experiments on eight selfconsolidating concrete slabs with both small and large openings. The results revealed that applying CFRP sheets significantly improved the slabs' load-carrying capacity by 46.67% for slabs with small openings and by 55.7% for those with large openings. Mohamed et al. [5] used ABAQUS software to carry out a numerical study on slabs with openings that were strengthened using CFRP. Their focus was on understanding how the thickness of the CFRP affected the results. The study showed that CFRP laminates greatly enhanced both the flexural capacity and overall stiffness of the slabs with openings. Furthermore, increasing the thickness of the CFRP laminates was found to significantly reduce midspan deflection. In the early 1990s, the use of GFRP rebars instead of steel rebars were investigated by many researchers as GFRP provide corrosion resistance, higher strength as well as its

lightweight property [12-16]. Due to the increase of shear lag between fibres, the tensile strength increases as the diameter of the bar increases generally, GFRP bars shows linear stressstrain behaviour up to rupture at the ultimate strength. Moreover, GFRP rebars do not experience strain hardening or yield deformation before reaching ultimate strength [17, 18]. Chang and Seo studied the structural behavior of reinforced concrete slabs with GFRP bars as internal reinforcement, there results indicated that the GFRP-reinforced slabs develop a bilinear elastic response up to the point of failure. With the linear elastic deformation of FRP and the concrete crushing failure mode that comes very often in FRP-reinforced slabs, it has been remarked that GFRP-reinforced slabs are much more flexible after cracking than steel ones. To achieve a deflectioncontrolled design, higher ratios of reinforcement had to be used. Thereby, this offers sufficient bending stiffness.

Elzaroug et al. [17] experimentally investigated three RC slabs reinforced with GFRP rebars. According to their type of reinforcement. From the test results, it is evident that though the GFRP reinforcement ratio of the test specimens was much higher than the balanced ratio, the slabs performed within the expected limit. The experiments by Attia et al. [19] were conducted on twelve concrete slabs reinforced with one-way BFRP and GFRP bars under a four-point load arrangement. It was observed that all slabs failed due to concrete crushing in the constant moment zone, and no FRP rupture failure was detected as evidenced by measured strain.

Shirmardi and Mohammadizadeh [20] conducted a numerical study to investigate the behavior of 20 concrete beams internally reinforced with GFRP bars. The findings demonstrated that the finite element model successfully simulated the flexural behavior of FRP-reinforced beams. Additionally, the results highlighted that FRP reinforcement is a viable solution for improving the ductility of reinforced concrete beam members.

Golham and Al-Ahmed [21] performed both experimental and numerical investigations to analyze the flexural behavior of one-way concrete slabs with openings, reinforced with GFRP bars and strengthened using CFRP sheets. Their findings revealed that CFRP sheets increased the flexural load-carrying capacity by approximately 52% and 44%, enhanced stiffness at ultimate load by about 101% and 95%, and reduced deflection under service loads by around 56% and 54%, respectively, compared to unstrengthened slabs. The specimens were also modeled using ABAQUS software, and a strong correlation was observed between the experimental and numerical results.

A few studies on a slab reinforced with GFRP bars have been conducted that investigate the effect of opening characteristics, and only a limited range of parameters were considered. In this research, the behavior of GFRP-reinforced concrete slabs with openings was extensively examined. The effect of opening shape, number, and location has been analyzed, and various technic to increase the slab strength were proposed, such as incorporating additional reinforcement bars and steel plates around the opening. These aspects were assessed by using finite element simulations.

2. FINITE ELEMENT SIMULATION

2.1 Element type, surface contact, and boundary condition

The FE models were generated in ABAQUS software

environment. The concrete was modelled using threedimensional hexahedral element while shell element was used to model for CFRP sheet and truss element was used for reinforcement. For the contact between the concrete and the CFRP sheets, Tie constraint was assigned while an embedded technique has been used between reinforcement and concrete. Displacement controlled technique has been used to apply the load on the loading plates. To simulate the experimental test pin-roller support condition was assigned in which the transition of one of the supports was restrained against moving in x and y direction while only x direction was restrained in the other one. Figure 1 presents the reference solid slab simulation.

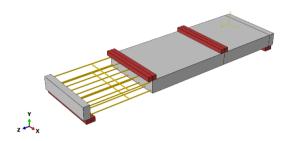


Figure 1. Modeling of the solid reference model (SS-WS)

2.2 Material constitutive models

2.2.1 Concrete

In Abaqus, defining material properties for concrete typically requires inputting both elastic and plastic characteristics. For the elastic behavior of concrete, which describes its response up to the cracking point (around $0.4\ fc'$, where fc' is the compressive strength), two independent parameters are essential: the elastic modulus (Ec) and Poisson's ratio (v). The elastic modulus is often calculated using guidelines like those from the ACI 318 code, with specific values typically summarized in Table 1.

For plastic behavior, the Concrete Damage Plasticity (CDP) model is a commonly used constitutive model designed for finite element analysis. The CDP model is based on plasticity and damage theory, as elaborated by several researchers [22, 23]. Using this model involves defining five key parameters and incorporating uniaxial compressive and tensile stress-strain relationships [24, 25]. These five parameters are biaxial to uniaxial compression ratio ($f_{bo}f_{co}$), shape factor (K_c), potential eccentricity (ξ), dilation angle (Ψ), and viscoplastic regularization parameter (μ). Parameters used for the concrete are presented in Table 1.

In addition, the concrete's uniaxial compressive behaviour was modelled utilizing a nonlinear stress-strain model proposed by Saenz [26], and the tensile stress-strain relationship was modelled by the approach presented by Hordijk [27].

Table 1. Plastic and elastic parameters of concrete defined in ABAOUS

			Cl	DP Paramo	eters			
E_c (MPa)	v	ξ	K_c	fbo/fco	μ	Ψ		
24870.06	0.3	0.1	0.667	1.16	0.0001	36		

2.2.2 GFRP and CFRP sheets

GFRP bars exhibit linear behavior up to failure. In this

study, GFRP bars were modelled using elastic-perfectly plastic behaver. The CFRP sheets are orthotropic elastic materials. Therefore "LAMINA" material type was used to model the elastic properties of CFRP sheets. The elastic modulus in the hoop direction E1 was found to be 230 GPa which was obtained from the manufacturer. Hashin damage model is used the plastic behavior of the CFRP sheets. The data for modelling GFRP and CFRP can be find in reference [21].

3. MODELS VALIDATION

To validate the numerical work, five models were simulated and calibrated against experimental work conducted in reference [21]. The specimens have the designation of SS, S1-WS, S2-WS, S1-FS, and S2-FS. Here, SS represents the solid slab, S1 refers to a slab with one opening, S2 for the slab with two openings, WS denotes the slab without strengthening, and FS refers to the strengthened slab.

Table 2. Comparison between the experimental and FEM ultimate strength

Specimen Designation	P _{u,exp} (kN)	P _{u,FM} (kN)	$P_{u,exp}/P_{u,FM}$	
SS	153	156	0.98	
S1-WS	90	96	0.94	
S2-WS	87	90	0.98	
S1-FS	137	140	0.99	
S2-FS	125	130	0.96	

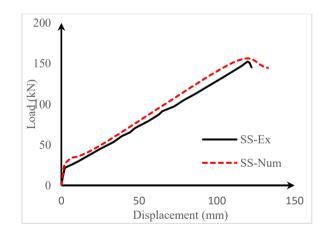


Figure 2. Load versus deflection curves for solid slab (SS)

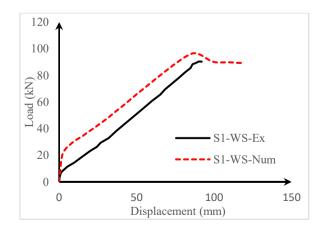


Figure 3. Load versus deflection curves for single opening slab without strengthening (S1-WS)

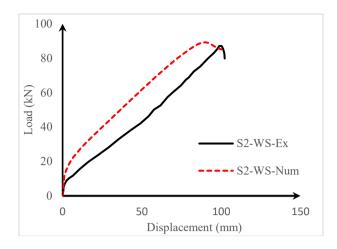


Figure 4. Load versus deflection curves for two opening slab without strengthening (S2-WS)

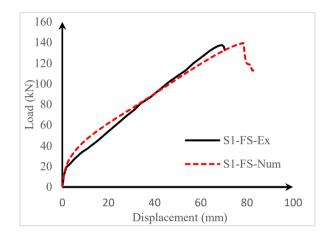


Figure 5. Load versus deflection curves for single opening slab strength with CFRP (S1-FS)

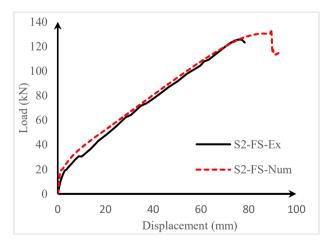


Figure 6. Load versus deflection curves for two opening slab strengthened with CFRP sheets (S1-FS)

The results found in reference [21] experimental work conducted were compared with the corresponding value reported in ABAQUS. First, the model's ultimate strengths were cross-checked as shown in Table 2. Second, the load-deflection curves of experimental and numerical work were correlated as presented through Figures 2-6. The Load-deflection curves reveal a good agreement between the experimental and numerical behavior. Furthermore, the ultimate load indicates reliability with a maximum difference of 7%. Finally, the cracks pattern is highlighted in Figure 7.

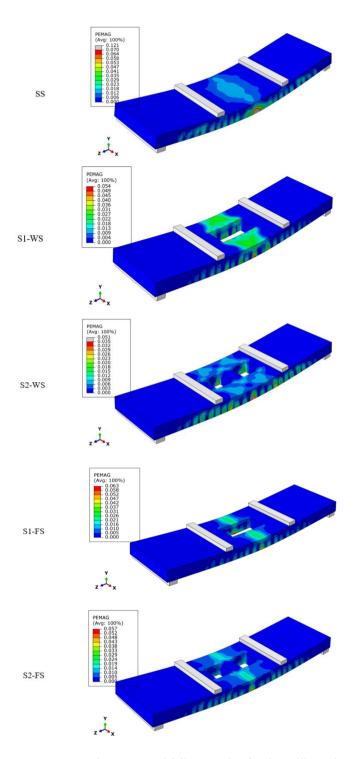


Figure 7. Crack pattern and failure modes for the calibrated models

4. PARAMETRIC STUDY

To extend the study of the opening effect, several parameters were considered by simulation additional slabs. Different opening shapes so that the one with the lowest impact on slab strength can be recommended. Furthermore, models with various opening numbers and locations were selected whether it was in mid-span or near supports, thus their effect on the slab flexural and shear strength can be evaluated. After that, and when the openings were unavoidable, several strengthening techniques were studied to determine the more efficient method.

The models were divided into three groups as indicated in Table 3. The first group included four slabs shape and the location of the opening was changed from a rectangular shape previously tested by the reference [21] to the elliptical shape. It includes two slabs where one slab had an elliptical shape opening (S1-WS-E), and the other slab had two openings (S2-WS-E) with elliptical shape as shown in Figure 8. In addition, it includes two slabs where the location of the opening was changed to be near the support (S1-WS-L and S2-WS-L) as shown Figure 9.

The second group included four models where two slabs were simulated based on Si-WS models, these two was strengthened using rebars around the opening (Si-WS-SB) as showed in Figure 10. and the other model was strengthened using steel plates planted near the opening (Si-WS-SP) as presented in Figure 11.

Third group includes two slabs strengthened with CFRP sheets and steel plates planted at slab top surface. The models were simulated based on S1-FS and S2-FS specimen, the slab strengthened with top steel plates has a designation of S1-FS-SP and S2-FS-SP presented in Figure 12.

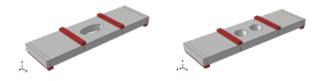


Figure 8. Slabs S1-WS-E and S2-WS-E

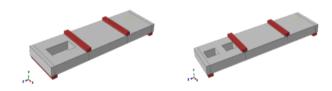


Figure 9. Slabs S1-WS-L and S2-WS-L

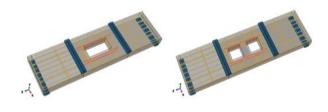


Figure 10. Additional rebars around openings S1-S and S2-S

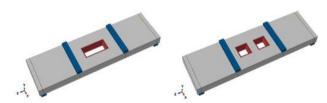


Figure 11. Slabs S1-SP and S2-SP

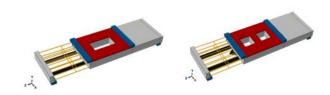


Figure 12. Slab S1-SP and S2-SP

Table 3. Slab's designation and its references

Group No.	Model Code	Opening Shape	Number of Openings	Opening Location	Additional Strengthening Type
	S1-WS-E	Elliptical	One	Centre	-
Group 1	S2-WS-E	Elliptical	Two	Centre	-
Group 1	S1-WS-L	Square	One	Near support	-
	S2-WS-L	Square	Two	Near support	-
	S1-SB1	Square	One	Centre	Steel bars around opening
Group 2	S2-SB2	Square	Two	Centre	Steel bars around opening
Group 2	S1-SP1	Square	One	Centre	Steel plates around opening
	S2-SP2	Square	Two	Centre	Steel plates around opening
Canada 2	S1-FS-SP1	Square	One	Centre	Steel plates on top of opening
Group 3	S2-FS-SP2	Square	Two	Centre	Steel plates on top of opening

5. RESULTS AND DISCUSSIONS

5.1 Effect of opening characteristics

5.1.1 Opening shape

It is predicted that round openings will tend to have a less detrimental effect on the slab's stiffness and load-bearing capacity compared to rectangular or irregular shapes. This is because the stress concentration around a circular opening is more evenly distributed, reducing the occurrence of cracking. However, larger circular openings can still significantly reduce the slab's stiffness and capacity. Using the reference models S1-WS and S2-WS simulated based on experimental work stated earlier, to determine the shape effect of the models S1-WS-E1 (one opening) and S2-WS-E1 (two openings) with elliptical opening. No large influence has been recorded by changing the opening shape of the current models as the failure mode was dominated by flexure and mainly depended on tensile bars and concrete compressive block. However, that conclusion is different from that reported by reference [28], their work was focused on steel reinforced concrete slab resistance of punching shear which the opening shape has a large effect on slab strength. The study conducted by reference [29] has a similar conclusion of shape effect, their study was on steel-reinforced concrete slab have an opening with different shapes, the circular opening record similar behavior of rectangular one, when the flexural failure mode is expected.

5.1.2 Opening location

Based on the results obtained from shafting the opening near the slab supports (S1-WS-L and S2-WS-L models), the slab flexural capacity and stiffness were increased. Hence, the opening near support could affect the shear strength nor bending resistance. As stated earlier, the main failure mode was in flexure, thus shifting the opening far from the point of maximum moment doesn't decrease the slab strength. The same conclusion was observed in references [28, 29]. In the current models, the stiffness for the slab with an opening near support increased by 24.58% in the elastic stage (K_{ela}) and 12.06% after cracking (K_{pla}) compared to the reference slab while load carrying increased by 19.79% in slab with only one opening compared to reference slab. The results showed that the change of opening's location of slab with two opening causes less enhancement compared to reference slab.

The load-displacement curves, which clarify the behavior of the opening effect either it was the shape influence or its location impact are presented in Figure 13 for slabs with one opening and Figure 14 for the slabs with two openings.

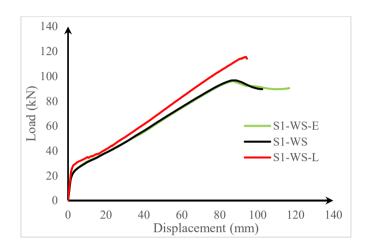


Figure 13. Load-deflection curve of slabs S1-WS-E, S1-WS-L and S1-WS which had one opening

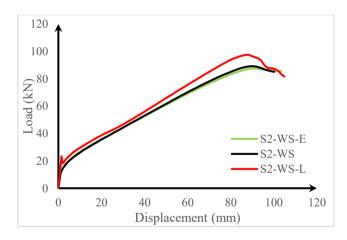


Figure 14. Load-deflection curve of slabs S2-WS-E, S2-WS-L and S2-WS which had two openings

5.2 Effect of add strengthening around the openings

5.2.1 General

Additional reinforcement or steel plates around openings counteract the stress concentration that naturally occurs around the edges of an opening which creates a stronger load path around the discontinuity and prevents the development and propagation of cracks around the opening. Therefore, it is anticipated that the addition of rebars around the opening will affect local Stiffness, and load-bearing capacity can offer better crack control. The impact of these two techniques in slab strengthening can be evaluated based on the behavior presented in the load-deflection curves. The strengthen slab with one opening (S1-SB, S1-SP), and its reference slab

without strengthening (S1-WS) are presented in Figure 15, while the load-deflection curves of S2-WS-SB, S2-WS-SP and its corresponding model without strengthening (S2-WS) are highlighted in Figure 16.

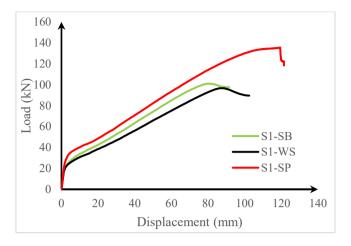


Figure 15. Load-deflection curve of slabs S1-WS-SB, S1-SP and S1-SB which had one opening

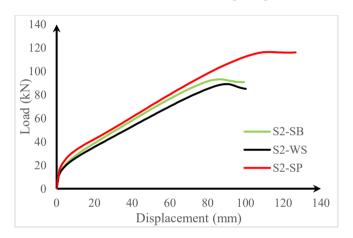


Figure 16. Load-deflection curve of slabs S2-WS, S2-SB and S2-SP which had two openings

5.2.2 Adding bars around opening

The addition of rebars increased the load-carrying capacity by 4.2%, where this enhancement relies on the additional steel reinforcement area that could minimize the cracks and add to the flexural capacity of the concrete, while no large influence has been recorded for slab stiffness. By splitting the slab opening from one to two with the same area, the additional bar presents more efficiency, which records an enhancement of 5.6%, as the bars are distributed around the opening and control a larger area in the concrete. This conclusion coincides with the ACI code recommendations of providing additional bars near the opening.

5.2.3 Adding steel plates around opening

The addition of plates around the opening shows the best results as the stiffness increased by 16.8% in the elastic stage and 2.8% after cracking compared to the reference model without strengthening. The load-carrying capacity increased up to 40.63% compared to the one-opening slab, as the steel plate can contribute with high efficiency to both the slab's compressive and tensile strength by creating a strong perimeter around the opening. The slab with two openings

(S2-SP) recorded less improvement in its ultimate strength, in which the enhancement was 31.1% by comparing it with the slab (S2-WS), the reason for that can be due to presence of the non-strengthened concrete region between the two openings, which expected to fail before the steel plate reach its full potential of strengthening.

5.3 Effect of the planting steel plates at slab top surface

Planting steel plates on top of the opening is predicted to increase the flexural capacity and stiffness. The slab (S1-FS and S2-FS) which have a CFRP sheet strengthening, were modeled as a reference for this group. The reference slabs failed by concrete compressive crushing. Therefore, a planted steel plate was simulated to increase the slab compressive strength in the models S1-FS-SP and S2-FS-SP.

The displacement-load curves extracted from the numerical analysis of these slabs are presented in Figure 17 and Figure 18. Planting the top plate for one opening slab increased the ultimate strength by 39.29% and the stiffness was increased by 72.5% before the occurrence of the crack and increased by 41.09% after cracking. However, the slabs including two openings present less enhancement as the flexural capacity increased by only 26.92%, this can be because high stress was noticed at the opening sides that are closer to the loading point. On the other hand, a similar stiffness was observed to that of the one-opening slab (Table 4).

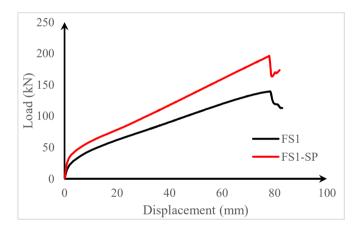


Figure 17. Load-deflection curve of slabs S1-FS and S1-FS-SP which had one opening

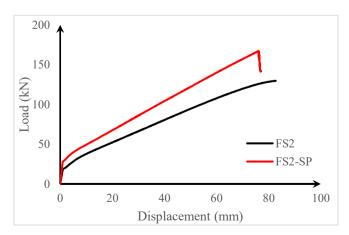


Figure 18. Load-deflection curve of slabs S2-FS and S2-FS-SP which had one opening

Table 4. Results summary

Slab Designation	K_{el}	K_{Pl}	P _{u (kN)}	$\frac{P_u}{P_{ref}}$	$\frac{K_{el}}{K_{el_{ref}}}$	$\frac{K_{pl}}{K_{pl_{ref}}}$
S1-WS (reference)	12	1.10	96	- rej	- el _{ref}	- pı _{ref}
S1-WS-E	12	1.10	96	0.00	0.00	0.00
S1-WS-L	15	1.24	115	19.7	24.6	12.1
S2-WS (reference)	11	1.05	90	-	-	_
S2-WS-E	11	1.05	90	0.0	0.0	0.0
S2-WS-L	11	1.09	95	5.6	0	4.3
S1-SB	12	1.27	100	4.2	0.0	14.7
S1-SP	14	1.13	135	40.6	16.8	2.8
S2-SB	11	1.12	95	5.6	0.0	6.8
S2-SP	11	1.07	118	31.1	3	2.5
S1-FS (reference)	17	1.79	140	-	-	-
S1-FS-SP	29	2.53	195	39.3	72.5	41.1
S2-FS (reference)	18	1.60	130	-	-	-
S2-FS-SP	31	2.23	165	26.9	72.8	38.9

6. CONCLUSIONS

This study investigated the behavior of one-way slabs reinforced with GFRP bars and having an opening. The effect of the opening shape and its location was determined based on the finite element. simulation conducted in this work. Furthermore, several techniques were proposed to enhance this kind of slab. First, the models were validated based on experimental tests reported in the literature. Then, the previously stated parameters were analyzed. The key finding of this study can be summarized as follows:

- 1) The finite element models developed in this work can capture the behavior of GFRP-reinforced concrete slabs with high accuracy.
- 2) When the failure modes are dominated by flexure, the effect of the opening shape either it was rectangle or elliptical, has no impact on slab behavior.
- 3) Shifting the opening away from points of maximum moment increases the stiffness by 24.58% and flexural strength by 12.06%, by assuming it has an adequate shear capacity near supports.
- 4) Providing steel plates around the openings, has been proven to be the most effective strengthening technique, as it improves the slab strength up to 40.63% in a single-opening slab, and 31.1% in a two-opening slab.
- 5) The concrete area between the two openings, is identified as the next weak point when this opening is strengthened with steel plates around its perimeter. This region could be further strengthened in future studies.
- 6) By planting a steel plate in the concrete compressive region, the improvement percentage in the slab's ultimate strength reached 39.29% for one opening slab and 26.9% for the two opening slabs. Which these slabs were already strengthened with CFRP sheets in its bottom fiber.

7. RECOMMENDATIONS FOR FUTURE STUDIES

Based on the findings and limitations of this research, the following points are recommended to investigation further for future studies:

- (1) Use high-strength concrete to clarify its impact on the model's compressive strength and its ultimate capacity.
- (2) Clarify the influence of the same parameters used in this study when the failure mode is dominated by shear.

- (3) Enhance the strength of the concrete region between the two openings in the slabs that have a steel plate around its opening.
- (4) Use different strength techniques to investigate its impact such as near-surface mounted (NSM), and BFRP sheets.

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